



## เอกสารอ้างอิง

1. ปรีชา แสงอาสภวิริยะ ภาวดี วรปัญญาวัฒนา วาสูแทน ฐะประสน และวีรยุทธ วงษ์ศิริ ภาษาคอมพิวเตอร์โปรแกรม 77 ภาควิชาคณิตศาสตร์ สถิติ คณะศิลปศาสตร์ มหาวิทยาลัยธรรมศาสตร์ พิมพ์ครั้งที่ 1, 2527
2. ชยาภิตย์ วัฒนาวรกิจ มาตรฐานการออกแบบสะพาน, หน้า 1-117. ภาควิชาวิศวกรรมศาสตร์ คณะวิศวกรรมศาสตร์ มหาวิทยาลัยขอนแก่น พิมพ์ ครั้งที่ 1 2525
3. ดำรงค์ เหมดี การวิเคราะห์โครงสร้าง, หน้า 4.1-5.23 ภาควิชาวิศวกรรมศาสตร์ คณะวิศวกรรมศาสตร์มหาวิทยาลัยแก่นพิมพ์ครั้งที่ 2 2526
4. Chu-kia Wang, Ph.d., Statically Indeterminate Structure, P 137 - 280 ,Tosho Printing Co Ltd., Tokyo, 1953
5. C.P.Heins and D.A Firmage, Design of Modern Steel highway Bridge, pp 1-154, John Wiley Sons, Inc,. New York, 1979
6. สันัน เจริญเผ่า และวินิต ช่อวิเชียร คอนกรีตเสริมเหล็ก, หน้า 39-193 ภาควิชาวิศวกรรมศาสตร์ คณะวิศวกรรมศาสตร์ จุฬาลงกรณ์มหาวิทยาลัย พิมพ์ครั้งที่ 6, 2527
7. George Winter And Arther H. Nilson, Design of Concrete Structures, pp 560 - 594. Tosho Printing Co.Ltd., tokyo, 1981
8. Conrad P. Heins and Richard A Lawrie, Design of Modern Concrete Highway Gridge, pp 132-203, A Wiley - Intersection Publication, New Youk, 1984.
9. James A. Moe, Ronald Reagan, J.E. Mc Mahon, R.J. Datel, Manaul of Bridge Design Practice, pp 1-100, 3 rd ed., 1971
10. Thomas Richard Mc. Calla, Introduction to Numerical Methods and Fortran Programming, pp 263 - 299. John Wiley Sons, Incl, New York, 1967.

11. Frederick W. Taylor, Sanford E. Thompson and Edward Smulski,  
Reinforced - Concrete Bridges, pp 194 - 216, John Wiley Sons,  
Inc., New York, 1955
12. เนียมศรี ลัดดาวัลย์ สูตรโครงสร้าง, หน้า 1-15, Cosmo Technological  
Consultants Co.Ltd., กรุงเทพฯ, นิคมสร้างที่ 1, 2528



ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย



ภาคผนวก ก.

โปรแกรมคอมพิวเตอร์

ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย



```

CALL HEAD ('                               and OUT3.TXT          ')
CALL PAGE (1)
CALL HEAD (' 3> QUIT to PRIME OPERATING SYSTEM                 ')
CALL PAGE (2)
CALL READI('                               SELECT NUMBER (1,2 or 3) : ',IWAY )
CALL PAGE (1)
GOTO (1,2,3), IWAY
  GOTO 100
1 CALL EDMENU
  GOTO 100
2 CALL COMPUTE
  GOTO 100
3 CALL PAGE (1)
CALL HEAD (' ***** END OF JOB ***** ')
CLOSE (3)
CLOSE (6)
CLOSE (11)
CLOSE (14)
CLOSE (18)
STOP
END

C
C
C *****
C *          BLOCK DATA          *
C *****
C
C          BLOCK DATA
C
C          COMMON /STESEC/ CR,ST(12,2),FORM(12,2)
C          COMMON /MKSBAR/ KDIA(12)
C          COMMON /REACTI/ REA
C
C          DATA CR /2.0/
C          DATA ST /0.00,0.25,0.375,0.50,0.625,0.75,0.875,1.00,
C          +   1.125,1.27,1.41,1.693,
C          *   0.00,0.05,0.11,0.20,0.31,0.44,0.60,0.79,
C          +   1.00,1.27,1.56,2.25/
C          DATA FORM /0.00,2.75,3.0,3.25,3.5,3.75,4.0,4.25,
C          +   4.5,4.75,5.0,5.5,
C          *   0.00,3.5,3.75,4.0,4.25,4.5,4.75,5.0,
C          +   5.25,5.5,6.0,7.0/
C          DATA KDIA /4.6,9,12,15,19,22,25,28,30,33,36/
C          DATA REA /0.0/
C          END

C
C *****
C *          SUB PROGRAM I          *
C *****
C
C          SUBROUTINE EDMENU
C
C          EDITING DATA OF (STEEL, CONCRETE, BRIDGE SPEC. )
C
C          CHARACTER HEADI*52

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INTEGER CHANGE, TYP
COMMON /TITLE/ HEADI
COMMON /MATER/ FCP,FSY
COMMON /LCLASS/ LCLASS
COMMON /STSIZE/ NOSB,NOST,NOSL
COMMON /BRGDIA/ NUMTRA,WLANE,EGDAP,NOBEAM,CSL,PAREA,NUMSPA
COMMON /PBEAM/ BWIDTH,SINTER
COMMON /BEAM/ TYP(9),SL(9),NSC(9),SL1(9),SL2(9),DO(9),D1(9),D2(9)
COMMON /SETTLE/ STM(10)
COMMON /LOAD/DD1,DE1,D11,DLL2,PLM,PLS
C START PROCESSING
C
REWIND 5
READ (5,1000) HEADI
READ (5,1100) FCP,FSY,WLANE,GDSPC
READ (5,1100) CSL,PAREA,EGDAP
READ (5,1200) NOSB,NOST,NOSL,LCLASS,NUMTRA,
+ NUMSPA,NOBEAM
REWIND 12
READ (12,1250) BWIDTH
READ (12,1250) SINTER
DO 10 I=1,9
10 READ (12,1300) TYP(I),SL(I),NSC(I),SL1(I),SL2(I),DO(I),D1(I),D2(I)
READ (12,1400) (STM(I),I=1,10)
C
C
C EDIT MENU
C
CHANGE = 0
100 CALL CLRSCR
CALL HEAD ( ' ***** )
CALL HEAD ( ' ***** EDITTING MENU ***** )
CALL HEAD ( ' ***** )
CALL PAGE (1)
CALL HEAD ( ' 1> Edit Title Heading )
CALL PAGE (1)
CALL HEAD ( ' 2> Edit Material Properties )
CALL PAGE (1)
CALL HEAD ( ' 3> Edit Load Class )
CALL PAGE (1)
CALL HEAD ( ' 4> Edit Bridge Dimensions )
CALL PAGE (1)
CALL HEAD ( ' 5> Edit Beams Dimensions )
CALL PAGE (1)
CALL HEAD ( ' 6> Edit Bars Size )
CALL PAGE (1)
CALL HEAD ( ' 7> Edit Settlement Size )
CALL PAGE (1)
CALL HEAD ( ' 8> <<< Return To Main-Menu >>> )
CALL PAGE (1)
CALL READI( ' SELECT NUMBER (1,2,....,8) : ',IWAY )
CALL PAGE (1)
GOTO (1,2,3,4,5,6,7,8),IWAY
GOTO 100
1 CHANGE = 1
CALL EDITI
GOTO 100
2 CHANGE = 1
CALL EDMAT
GOTO 100

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3  CHANGE = 1
   CALL EDLDC
   GOTO 100
4  CHANGE = 1
   CALL EDDIM
   GOTO 100
5  CHANGE = 1
   CALL EDBMD
   GOTO 100
6  CHANGE = 1
   CALL EDBAR
   GOTO 100
7  CHANGE = 1
   CALL EDSTM
   GOTO 100
8  IF (CHANGE .EQ. 1) THEN
   REWIND 5
   WRITE(5,1000) HEADI
   WRITE(5,1100) FCP,FSY,WLANE,GDSPC
   WRITE(5,1100) CSL,PAREA,EGDAP
   WRITE(5,1200) NOSB,NOST,NOSL,LCLASS,NUNTRA,
+      NUMSPA,NOBEAM
   REWIND 12
   WRITE(12,1250) DWIDTH
   WRITE(12,1250) SINTER
   DO 20 I=1,9
20  WRITE(12,1300) TYP(I),SL(I),NSC(I),SL1(I),SL2(I),DO(I),D1(I),D2(I)
   WRITE(12,1400) (STM(I),I=1,10)
   ENDIF

C
C
C
1000 FORMAT (A)
1100 FORMAT (8F10.3)
1200 FORMAT (16I5)
1250 FORMAT (F10.4)
1300 FORMAT (15,F10.4,15,5F10.4)
1400 FORMAT (10F10.3)
   RETURN
   END

C
C
C
      -----
      SUBROUTINE  EDITIT
      -----

C
C
EDIT TITLE OF FILE # 5
CHARACTER HEADI*52
COMMON /TITLE/ HEADI
100 CALL CLRSCR
   CALL PAGE (1)
   CALL HEAD (' ***** ')
   CALL HEAD (' ***** TITLE HEADING EDIT ***** ')
   CALL HEAD (' ***** ')
   CALL PAGE (2)
   CALL HEAD (' An Existing Title is shown below ')
   CALL PAGE (2)
   CALL HEAD (HEADI)
   CALL PAGE (2)
   CALL HEAD (' 1> ENTER THE NEW TITLE ')
   CALL PAGE (1)

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CALL HEAD ('          2> EVERYTHING IS ALLRIGHT          ')
CALL PAGE (2)
CALL READI('          SELECT NUMBER ( 1 or 2 ) : ',IWAY )
CALL PAGE (1)
GOTO (1,2) ,IWAY
  GOTO 100
1 CALL HEAD (' Enter the new Title you want to refer to later.... ')
CALL PAGE (1)
CALL READC(' Enter Char. : ',HEAD1)
  GOTO 100
P RETURN
END
C
C
C          -----
C          SUBROUTINE  EDMAT
C          -----
C
C EDIT MATERIAL PROPERTIES
C
C      COMMON /MATER/ FCP,FSY
C
C 100 CALL CLRSCR
CALL HEAD (' ***** ')
CALL HEAD (' ***** MATERIAL PROPERTIES ***** ')
CALL HEAD (' ***** ')
CALL PAGE (2)
CALL FTOM (FCP,ZZ,6)
CALL LISTR(' 1> Ultimate Strength of concrete = ',ZZ, ' ksc. ')
CALL PAGE (1)
CALL FTOM (FSY,ZZ,6)
CALL LISTR(' 2> Yield Strength of Steel = ',ZZ, ' ksc. ')
CALL PAGE (1)
CALL HEAD (' 3> EVERYTHING IS ALLRIGHT          ')
CALL PAGE (2)
CALL READI('          SELECT NUMBER (1,2 or 3) : ',IWAY )
CALL PAGE (1)
GOTO (1,2,3) ,IWAY
  GOTO 100
1 CALL READR(' Ultimate Strength of Concrete (ksc.) = ',ZZ )
CALL MTOF (ZZ,FCP,6)
  GOTO 100
2 CALL READR(' Yield Strength of Steel (ksc.) = ',ZZ )
CALL MTOF (ZZ,FSY,6)
  GOTO 100
3 RETURN
END
C
C
C          -----
C          SUBROUTINE  EDLDC
C          -----
C
C EDIT LOAD CLASS
C
C      COMMON /LCLASS/ LCLASS
C
C 100 CALL CLRSCR
CALL PAGE (1)
CALL HEAD (' ***** ')
CALL HEAD (' ***** LOAD CLASS EDITING ***** ')
CALL HEAD (' ***** ')

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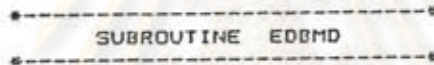
CALL PAGE (2)
IF (LCLASS .EQ. 1) GOTO 110
IF (LCLASS .EQ. 2) GOTO 120
  GOTO 200
110 CALL HEAD (' =====> by now Load Class is HS20 ')
CALL PAGE (2)
  GOTO 200
120 CALL HEAD (' =====> by now Load Class is HS15 ')
CALL PAGE (2)
  GOTO 200
200 CALL HEAD (' 1> HS20 Load Class ')
CALL PAGE (1)
CALL HEAD (' 2> HS15 Load Class ')
CALL PAGE (1)
CALL HEAD (' 3> EVERYTHING IS ALLRIGHT ')
CALL PAGE (2)
CALL READI(' SELECT NUMBER (1,2 or 3) : ', IWAY )
CALL PAGE (1)
IF ((IWAY.NE.1).AND.(IWAY.NE.2)) GOTO 300
LCLASS = IWAY
  GOTO 100
300 RETURN
END
C
C
C -----
C SUBROUTINE EDDIM
C -----
C
C EDIT BRIDGE DIMENSIONS
C
COMMON /BRGDIA/ NUMTRA,WLANE,EGDAP,NOBEAM,CSL,PAREA,NUMSPA
C
100 CALL CLRSCR
CALL HEAD (' ***** ')
CALL HEAD (' ***** BRIDGE DIMENSION EDITING ***** ')
CALL HEAD (' ***** ')
CALL PAGE (1)
CALL LISTI(' 1> Number of Traffic Lane ', numtra, ' ')
CALL PAGE (1)
CALL FTOM (WLANE, ZZ, 4)
CALL LISTR(' 2> Lane Width ', zz, ' m. ')
CALL PAGE (1)
CALL FTOM (EGDAP, ZZ, 4)
CALL LISTR(' 3> Outer Dimension Appart Ex-Gir. ', zz, ' m. ')
CALL PAGE (1)
CALL LISTI(' 4> Number of Girder per section ', nobeam, ' ')
CALL PAGE (1)
CALL FTOM (CSL, ZZ, 4)
CALL LISTR(' 5> Overhanging Slab from Girder ', zz, ' m. ')
CALL PAGE (1)
CALL FTOM (PAREA, zz, 9)
CALL LISTR(' 6> Area of Parapet Family ', zz, ' sq.m. ')
CALL PAGE (1)
CALL LISTI(' 7> Number of span ', NUMSPA, ' ')
CALL PAGE (1)
CALL FTOM (RR, ZZ, 9)
CALL LISTR(' 8> Area of Curb ', ZZ, ' sq.m. ')
CALL PAGE (1)
CALL FTOM (CB, ZZ, 9)
CALL LISTR(' 9> Area of cross beam ', ZZ, ' sq.m. ')
CALL PAGE (1)
CALL HEAD (' 10> EVERYTHING IS ALLRIGHT ')
CALL PAGE (1)
CALL READI(' SELECT NUMBER (1,2,...,10) : ', IWAY )
CALL PAGE (1)
GOTO (1, 2, 3, 4, 5, 6, 7, 8, 9, 10), IWAY

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GOTO 100
1 CALL READR('          Number of Traffic Lane = ',NUMTRA)
  GOTO 100
2 CALL READR('          Lane Width (m.) = ',ZZ )
  CALL MTOF(ZZ,WLANE,4)
  GOTO 100
3 CALL READR(' Outer dimension appart EX-Girder (m.) = ',ZZ )
  CALL MTOF(ZZ,EGDAP,4)
  GOTO 100
4 CALL READR('          Number of Girder per section = ',NOBEAM)
  GOTO 100
5 CALL READR(' Overhang Slab from c.g. of Girder (m.) = ',ZZ )
  CALL MTOF(ZZ,CSL,4)
  GOTO 100
6 CALL READR(' Cross section Area of Parapet (sq.m.) = ',ZZ )
  CALL MTOF(ZZ,PAREA,9)
  GOTO 100
7 CALL READR('          Total Number of Span = ',NUMSPA)
  GOTO 100
8 CALL READR(' Curb area (sq.m.) = ',ZZ )
  CALL MTOF(ZZ,RR,9)
  GOTO 100
9 CALL READR(' Cross beam area (sq.m.) = ',ZZ )
  CALL MTOF(ZZ,CB,9)
  GOTO 100
10 RETURN
    END
  
```

C  
C  
C  
C  
C  
C  
C  
C  
C



EDIT BEAM DIMENSIONS

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COMMON /BRCDIA/ NUMTRA,WLANE,EGDAP,NOBEAM,CSL,PAREA,NUMSPA
COMMON /PBEAM/ BWIDTH,SINTER
  
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C

```

100 CALL CLRSCR
    CALL HEAD (' *****')
    CALL HEAD (' ***** BEAM DIMENSION EDITING *****')
    CALL HEAD (' *****')
    CALL PAGE(2)
    CALL FTOM(BWIDTH,ZZ,4)
    CALL LISTR('          1> Beam Width = ',ZZ, ' m. ')
    CALL PAGE(1)
    CALL FTOM(SINTER,ZZ,4)
    CALL LISTR('          2> Length of Interval = ',ZZ, ' m. ')
    CALL PAGE(1)
    CALL HEAD ('          3> EDIT EACH SPAN ')
    CALL PAGE(1)
    CALL HEAD ('          4> EVERY THING IS ALLRIGHT ')
    CALL PAGE(2)
    CALL READR('          SELECT NUMBER (1,2,...,4) : ',IWAY )
    CALL PAGE(1)
    GOTO (1,2,3,4),IWAY
    GOTO 100
1 CALL READR('          Beam Width (m.) = ',ZZ )
  CALL MTOF(ZZ,BWIDTH,4)
  GOTO 100
2 CALL READR(' Length of Segment Divides into (m.) = ',ZZ )
  CALL MTOF(ZZ,SINTER,4)
  GOTO 100
3 CALL EDBMDS(NUMSPA)
  GOTO 100
  
```

```

4 RETURN
END
C
C
C-----*
SUBROUTINE EDBMDS(NUMSPA)
C-----*
200 CALL CLRSCR
CALL HEAD (' ***** ')
CALL HEAD (' ***** BEAM DIMENSION EDITING ***** ')
CALL HEAD (' ***** ')
CALL PAGE (2)
DO 80 I = 1, NUMSPA
WRITE (*, 1000) I, I
1000 FORMAT (16X, I3, '> Edit Span No. ', I2)
80 CALL PAGE (1)
WRITE (*, 1100) I
1100 FORMAT (16X, I3, '> EVERYTHING IS ALLRIGHT ')
CALL PAGE (1)
CALL READI(' SELECT NUMBER (1,2,3,...) : ', IWAY )
IF (IWAY.GE.1) GOTO 11
CALL PAGE (1)
CALL DEPTH(IWAY)
GOTO 200
11 RETURN
END
C
C
C-----*
SUBROUTINE DEPTH(I)
C-----*
C
INTEGER TYP
COMMON /PBEAM/ BWIDTH, SINTER
COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), DO(9), D1(9), D2(9)
C
300 CALL CLRSCR
CALL HEAD (' ***** ')
WRITE (*, 333) I
333 FORMAT(14X, ' ***** SPAN NUMBER ', I2, ' *****
**')
CALL HEAD (' ***** ')
CALL PAGE (2)
CALL LISTJ(' ==> by now Code of Beam Shape = ', TYP(I) )
CALL PAGE (2)
CALL HEAD (' Code : 1 = Straight haunch ')
CALL PAGE (1)
CALL HEAD (' 2 = Parabolic haunch ')
CALL PAGE (1)
CALL HEAD (' 3 = Constant Cross Section ')
CALL PAGE (1)
CALL HEAD (' Command : 4 > CONTINUE ')
CALL PAGE (1)
CALL HEAD (' 5 > RETURN ')
CALL PAGE (2)
CALL READI(' SELECT NUMBER (1,2,...,5) : ', IWAY )
CALL PAGE (1)
GOTO (1,2,3,4,5), IWAY
GOTO 300
1 TYP(I) = IWAY

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GOTO 300
2 TYP(I) = IWAY
GOTO 300
3 TYP(I) = IWAY
GOTO 300
4 GOTO (400, 400, 500), TYP(I)
5 RETURN

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C  
C  
C

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400 CALL CLRSCR
CALL HEAD ( ' ***** ' )
WRITE (*, 444) I
444 FORMAT(14X, ' ***** SPAN NUMBER ', IZ, ' *****
*)
CALL HEAD ( ' ***** ' )
CALL PAGE (1)
CALL FTOM (SL(I), ZZ, 4)
CALL LISTR(' 1> Span Length = ', ZZ, ' m. ')
CALL PAGE (1)
IZZ = NSC(I)
CALL LISTI(' Number of Section Consider = ', IZZ, ' sect. ')
CALL PAGE (1)
CALL FTOM (D1(I), ZZ, 4)
CALL LISTR(' 2> Left side Depth = ', ZZ, ' m. ')
CALL PAGE (1)
CALL FTOM (DO(I), ZZ, 4)
CALL LISTR(' 3> Centre Depth = ', ZZ, ' m. ')
CALL PAGE (1)
CALL FTOM (D2(I), ZZ, 4)
CALL LISTR(' 4> Right side Depth = ', ZZ, ' m. ')
CALL PAGE (1)
CALL FTOM (SL1(I), ZZ, 4)
CALL LISTR(' 5> Left side Length = ', ZZ, ' m. ')
CALL PAGE (1)
CALL FTOM (SL2(I), ZZ, 4)
CALL LISTR(' 6> Right side Length = ', ZZ, ' m. ')
CALL PAGE (1)
CALL HEAD (' 7> EVERY THING IS ALLRIGHT ')
CALL PAGE (1)
CALL READI(' SELECT NUMBER (1,2,...,7) : ', IWAY )
CALL PAGE (1)
GOTO (11, 12, 13, 14, 15, 16, 17), IWAY
GOTO 400
11 CALL READR(' Span Length (m.) = ', ZZ )
CALL HTOF (ZZ, SL(I), 4)
NSC(I) = SL(I)/SINTER + 1.5
GOTO 400
12 CALL READR(' Left side Depth (m.) = ', ZZ )
CALL HTOF (ZZ, D1(I), 4)
GOTO 400
13 CALL READR(' Centre Depth (m.) = ', ZZ )
CALL HTOF (ZZ, DO(I), 4)
GOTO 400
14 CALL READR(' Right side Depth (m.) = ', ZZ )
CALL HTOF (ZZ, D2(I), 4)
GOTO 400
15 CALL READR(' Left side Length (m.) = ', ZZ )
CALL HTOF (ZZ, SL1(I), 4)
GOTO 400

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```

16 CALL READR('          Right side Length (m.) = ',ZZ  )
   CALL MTOF (ZZ,SL2(I),4)
   GOTO 400
17 RETURN
C
C
500 CALL CLRSCR
   CALL HEAD (' *****')
   WRITE (*,555) I
555 FORMAT(14X,' ***** SPAN NUMBER ',I2,' *****
**')
   CALL HEAD (' *****')
   CALL PAGE (2)
   CALL FTOM (SL(I),ZZ,4)
   CALL LISTR(' 1> Span Length           = ',ZZ, ' m. ')
   CALL PAGE (1)
   IZZ = NSC(I)
   CALL LISTI('          Number of Section Consider = ',IZZ, ' sect. ')
   CALL PAGE (1)
   CALL FTOM (D1(I),ZZ,4)
   CALL LISTR(' 2> Depth of Beam           = ',ZZ, ' m. ')
   CALL PAGE (1)
   CALL HEAD (' 3> EVERY THING IS ALLRIGHT           ')
   CALL PAGE (2)
   CALL READI('          SELECT NUMBER (1,2,...,3) : ',IWAY )
   CALL PAGE (1)
   GOTO (21,22,23), IWAY
   GOTO 500
21 CALL READR('          Span Length           (m.) = ',ZZ  )
   CALL MTOF (ZZ,SL(I),4)
   SL1(I) = SL(I)
   SL2(I) = SL(I)
   NSC(I) = SL(I)/SINTER + 1.5
   GOTO 500
22 CALL READR('          Depth of Beam           (m.) = ',ZZ  )
   CALL MTOF (ZZ,D1(I),4)
   DO(I)=D1(I)
   DP(I)=D1(I)
   GOTO 500
23 RETURN
   END

```

C  
C  
C  
C

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-----
SUBROUTINE  EDBAR
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```

C
C
C EDIT SIZE OF STEEL BAR
COMMON /STSIZE/ NOSB,NOST,NOSL
100 CALL CLRSCR
   CALL HEAD (' *****')
   CALL HEAD (' ***** BAR SIZE EDITING *****')
   CALL HEAD (' *****')
   CALL PAGE (2)
   CALL LSTSTS
   CALL PAGE (2)
   CALL LISTI(' 1> BEAM Bar Size           # ',NOSB , ' ')
   CALL PAGE (1)
   CALL LISTI(' 2> BEAM Stirrup Bar Size        # ',NOST , ' ')
   CALL PAGE (1)

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CALL LISTI(' 3> SLAB Bar Size # ',NOSL , ' ')
CALL PAGE (1)
CALL HEAD (' 4> EVERYTHING IS ALLRIGHT ')
CALL PAGE (2)
CALL READI(' SELECT NUMBER (1,2,...,4) : ',IWAY )
CALL PAGE (1)
GOTO (1,2,3,4), IWAY
GOTO 100
1 CALL READI(' BEAM Bar Size # ',NOSB )
GOTO 100
2 CALL READI(' BEAM Stirrup Bar Size # ',NOST )
GOTO 100
3 CALL READI(' SLAB Bar Size # ',NOSL )
GOTO 100
4 RETURN
END
C
C-----
C LIST BARS DIAMETER
C-----
SUBROUTINE LSTSTS
COMMON /MKSDAR/ KDIA(12)
WRITE (*,2000) (I, I=1, 12)
WRITE (*,2010) (KDIA(I), I=1, 12)
2000 FORMAT (' Bar #', I2I5)
2010 FORMAT (' Diam. (mm.)', I2I5)
RETURN
END
C
C
C-----*
C SUBROUTINE EDSTM
C-----*
C
COMMON /BRGDIA/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
COMMON /SETTLE/ STM(10)
100 CALL CLRSCR
CALL HEAD (' ***** ')
CALL HEAD (' ***** SETTLEMENT EDITING ***** ')
CALL HEAD (' ***** ')
CALL PAGE (2)
DO 80 I = 1, NUMSPA+1
CALL FTOM (STM(I), ZZ, 4)
WRITE (*,1000) I, I, ZZ
1000 FORMAT (16X, I3, '> Settlement at Joint # ', I2, ' = ', FB 2, ' m. ')
80 CALL PAGE (1)
WRITE (*,1100) I
1100 FORMAT (15X, I3, '> EVERYTHING IS ALLRIGHT ')
CALL PAGE (1)
CALL READI(' SELECT NUMBER (1,2,3,...) : ', IWAY )
IF (IWAY.GE. I) GOTO 11
CALL PAGE (1)
CALL READR(' Settlement at this joint = ', ZZ )
CALL MTOF (ZZ, STM(IWAY), 4)
GOTO 100
11 RETURN
END
C
C

```



```

C
C
C      *-----*
C      SUBROUTINE READI (LABEL, IVAR)
C      *-----*
C
C      Read Integer in one line
C
C      CHARACTER*45 LABEL
10    CALL TNOVA (LABEL, INTS(45))
      READ (*,*,ERR=10) IVAR
      RETURN
      END
C
C
C      *-----*
C      SUBROUTINE READR (LABEL, RVAR)
C      *-----*
C
C      Read Real in one line
C
C      CHARACTER*45 LABEL
10    CALL TNOVA (LABEL, INTS(45))
      READ (*,*,ERR=10) RVAR
      RETURN
      END
C
C
C      *-----*
C      SUBROUTINE FTOM (FOOT, XMETRE, NO)
C      *-----*
C
C      Convert English Unit to SI Unit
C      NO.  1) in.  -> cm.
C           2) in.  -> m.
C           3) ft.  -> cm.
C           4) ft.  -> m.
C           5) lb.  -> kg.
C           6) psi  -> ksc.
C           7) psf  -> ksm.
C           8) sq.in. -> sq.cm.
C           9) sq.in. -> sq.m.
C          10) ft-kip -> kg-m.
C          11) kips -> kg.
C
C      -----
C      DIMENSION C(11)
C      DATA C /2.54,.0254,30.48,.3048,.4536,.0703,4.8025,6.4516,
C      + .000645,138.2573,453.6/
C      XMETRE = FOOT * C(NO)
C      RETURN
C      END
C
C
C      *-----*
C      SUBROUTINE MTOF (XMETRE, FOOT, NO)
C      *-----*
C
C      CONVERSE METRE TO FOOT
C
C      DIMENSION C(11)
C      DATA C /2.54,.0254,30.48,.3048,.4536,.0703,4.8025,6.4516,

```



```

+ .000645, 138. 2573, 453. 6/
FOOT = XMETRE / C(NO)
RETURN
END

C
C
C
C -----
C SUBROUTINE LISTR (FIRST, RVAR, LAST)
C -----
C
C LIST REAL IN ONE LINE
C
C CHARACTER FIRST*36, LAST*6
WRITE (*, 2000) FIRST, RVAR, LAST
2000 FORMAT (14X, A36, F10. 2, A6)
RETURN
END

C
C
C
C -----
C SUBROUTINE LISTI (FIRST, IVAR, LAST)
C -----
C
C LIST INTEGER IN 1 LINE
C
C CHARACTER FIRST*36, LAST*6
WRITE (*, 2000) FIRST, IVAR, LAST
2000 FORMAT (14X, A36, I10, A6)
RETURN
END

C
C
C
C -----
C SUBROUTINE LISTJ (FIRST, JVAR)
C -----
C
C LIST INTEGER IN 1 LINE
C
C CHARACTER FIRST*42
WRITE (*, 2000) FIRST, JVAR
2000 FORMAT (14X, A42, I2)
RETURN
END

C
C
C
C
C
C
C
C
C
C *****
C * SUB PROGRAM 2 *
C * COMPUTE CONSTANTS AND CALCULATION MENU *
C *****
C

```

```

C
C          -----
C          SUBROUTINE      COMPUTE
C          -----
C
C ANALYSIS AND DESIGN BRIDGE VARY CROSS SECTION
C
C      INTEGER      CHANGE, TYP
C      CHARACTER   HEADI*52
C      COMMON /TITLE/ HEADI
C      COMMON /MATER/ FCP, FSY
C      COMMON /STSIZE/ NOSB, NOST, NOSL
C      COMMON /BRGDIA/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
C      COMMON /LCLASS/ LCLASS
C      COMMON /SETTLE/ STM(10)
C      COMMON /PREAM/ BWIDTH, SINTER
C      COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D1(9), D2(9)
C
C      COMMON /RDEPTH/ BDEPTH
C      COMMON /CDATA/ FC, FS, MR, CK, CJ, CRR, VMAX, VCON1, VCON2, GDSPC
C      COMMON /LOAD /DD1, DE1, D11, DLL2, PLM, PLS
C      REWIND 5
C      READ (5, 1000) HEADI
C      READ (5, 1100) FCP, FSY, WLANE, GDSPC
C      READ (5, 1100) CSL, PAREA, EGDAP
C      READ (5, 1200) NOSB, NOST, NOSL, LCLASS, NUMTRA,
C      NUMSPA, NOBEAM
C      REWIND 12
C      READ (12, 1250) BWIDTH
C      READ (12, 1250) SINTER
C      DO 10 I=1, 9
10    READ (12, 1300) TYP(I), SL(I), NSC(I), SL1(I), SL2(I), D0(I), D1(I), D2(I)
      READ (12, 1400) (STM(I), I=1, 10)
C
C      1000 FORMAT (A)
C      1100 FORMAT (BF10.3)
C      1200 FORMAT (16I5)
C      1250 FORMAT (F10.4)
C      1300 FORMAT (I5, F10.4, I5, 3F10.4)
C      1400 FORMAT (10F10.3)
C
C
C      CALL WRITED
C
C      CALL CONLOAD
C
C      CALL MENCAL
C
C
C      RETURN
C      END
C
C          -----
C          SUBROUTINE      WRITED
C          -----
C
C WRITE TO FILE : OUT1.TXT

```

```

INTEGER    CHANGE, TYP
CHARACTER  HEADI*52
COMMON /TITLE/ HEADI
COMMON /MATER/ FCP, FSY
COMMON /STSIZE/ NOSB, NOST, NOSL
COMMON /BRGDIA/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
COMMON /LCLASS/ LCLASS
COMMON /SETTLE/ STM(10)
COMMON /PBEAM/ BWIDTH, SINTER
COMMON /DEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), DO(9), D1(9), D2(7)

```

C

```

COMMON /BDEPTH/ BDEPTH
COMMON /CDATA/ FC, FS, MR, CK, CJ, CRR, VMAX, VCON1, VCON2, QDPC

```

C

REWIND 6

C

```

CALL WRITER
CALL WRTL (4)
CALL WRTH (' ***** ')
CALL WRTH (' ***** TITLE HEADING LIST ***** ')
CALL WRTH (' ***** ')
CALL WRTL (2)
CALL WRTH ('           An Existing Title is shown below           ')
CALL WRTL (2)
CALL WRTH (HEADI)
CALL WRTL (2)
CALL WRTH ('-----')
CALL WRTL (4)

```

C

C

```

CALL WRTL (4)
CALL WRTH (' ***** ')
CALL WRTH (' ***** MATERIAL PROPERTIES ***** ')
CALL WRTH (' ***** ')
CALL WRTL (2)
CALL FTOM (FCP, ZZ, 6)
CALL WRTR (' 1> Ultimate Strength of concrete = ', ZZ, ' ksc. ')
CALL WRTL (1)
CALL FTOM (FSY, ZZ, 6)
CALL WRTR (' 2> Yield Strength of Steel = ', ZZ, ' ksc. ')
CALL WRTL (2)
CALL WRTH ('-----')
CALL WRTL (4)

```

C

C

```

CALL WRTL (4)
CALL WRTH (' ***** ')
CALL WRTH (' ***** LOAD CLASS LISTING ***** ')
CALL WRTH (' ***** ')
CALL WRTL (2)
IF (LCLASS .EQ. 1) GOTO 10
IF (LCLASS .EQ. 2) GOTO 20
  GOTO 30
10 CALL WRTH (' =====> by now Load Class is HS20 ')
  CALL WRTL (2)
  GOTO 30
20 CALL WRTH (' =====> by now Load Class is HS15 ')
  CALL WRTL (2)
  GOTO 30
30 CALL WRTH ('-----')

```

```

CALL WRITEP
CALL WRTL (4)
CALL WRTH (' ***** ' )
CALL WRTH (' ***** BRIDGE DIMENSION LISTING ***** ' )
CALL WRTH (' ***** ' )
CALL WRTL (1)
CALL WRTI (' 1> Number of Traffic Lane           ', NUMTRA, ' ' )
CALL WRTL (1)
CALL FTOM (WLANE, ZZ, 4)
CALL WRTR (' 2> Lane Width                       ', ZZ, ' m. ' )
CALL WRTL (1)
CALL FTOM (EGDAP, ZZ, 4)
CALL WRTR (' 3> Outer Dimension Appart Ex-Gir. ', ZZ, ' m. ' )
CALL WRTL (1)
CALL WRTI (' 4> Number of Girder per section ', NOBEAM, ' ' )
CALL WRTL (1)
CALL FTOM (CSL, ZZ, 4)
CALL WRTR (' 5> Overhanging Slab from Girder ', ZZ, ' m. ' )
CALL WRTL (1)
CALL FTOM (PAREA, ZZ, 9)
CALL WRTR (' 6> Area of Parapat Family           ', ZZ, ' sq.m. ' )
CALL WRTL (1)
CALL WRTI (' 7> Number of span                   ', NUMSPA, ' ' )
CALL WRTL (1)
CALL FTOM (RR, ZZ, 9)
CALL WRTR (' 8> Curb area                         ', ZZ, ' sq.m. ' )
CALL WRTL (1)
CALL FTOM (CB, ZZ, 9)
CALL WRTR (' 9> Cross beam area                  ', ZZ, ' sq.m. ' )
CALL WRTL (2)
CALL WRTH (' ----- ' )
CALL WRTL (4)

```

C  
C

```

CALL WRTL (4)
CALL WRTH (' ***** ' )
CALL WRTH (' ***** BEAM DIMENSION LISTING ***** ' )
CALL WRTH (' ***** ' )
CALL WRTL (2)
CALL FTOM (BWIDTH, ZZ, 4)
CALL WRTR (' 1> Beam Width = ', ZZ, ' m. ' )
CALL WRTL (1)
CALL FTOM (SINTER, ZZ, 4)
CALL WRTR (' 2> Length of Interval = ', ZZ, ' m. ' )
CALL WRTL (2)
CALL WRTH (' ----- ' )
CALL WRTL (4)

```

C

```

CALL WRITEP
CALL WRTL (5)
DO 50 I=1, NUMSPA
  IF (TYP(I).EQ.3) THEN
CALL WRTH (' ***** ' )
WRITE (6, JJJ) I
CALL WRTH (' ***** ' )
CALL WRTL (2)
CALL WRTJ (' =====> by now code of beam shape =', TYP(I) )
CALL WRTL (2)
CALL WRTH (' code : 1 = straight haunch ' )
CALL WRTL (1)
CALL WRTH (' 2 = parabolic haunch ' )
CALL WRTL (1)
CALL WRTH (' 3 = constant cross section ' )

```

CALL WRTL (4)

C  
C

```

CALL WRITEP
CALL WRTL (4)
CALL WRTH (' ***** ')
CALL WRTH (' ***** BRIDGE DIMENSION LISTING ***** ')
CALL WRTH (' ***** ')
CALL WRTL (1)
CALL WRTI (' 1> Number of Traffic Lane          ',numtra,' ')
CALL WRTL (1)
CALL FTOM (WLANE,ZZ,4)
CALL WRTR (' 2> Lane      Width          ',zz,' m. ')
CALL WRTL (1)
CALL FTOM (EGDAP,ZZ,4)
CALL WRTR (' 3> Outer Dimension Appart Ex-Gir.',zz,' m. ')
CALL WRTL (1)
CALL WRTI (' 4> Number of Girder per section ',nobeam,' ')
CALL WRTL (1)
CALL FTOM (CSL,ZZ,4)
CALL WRTR (' 5> Overhanging Slab from Girder ',zz,' m. ')
CALL WRTL (1)
CALL FTOM (PAREA,zz,9)
CALL WRTR (' 6> Area of Parapet Family          ',zz,' sq. m. ')
CALL WRTL (1)
CALL WRTI (' 7> Number of span          ',NUMSPA,' ')
CALL WRTL (2)
CALL WRTH ('-----')
CALL WRTL (4)

```

C  
C

```

CALL WRTL (4)
CALL WRTH (' ***** ')
CALL WRTH (' ***** BEAM DIMENSION LISTING ***** ')
CALL WRTH (' ***** ')
CALL WRTL (2)
CALL FTOM (BWIDTH,ZZ,4)
CALL WRTR (' 1> Beam      Width          = ',ZZ,' m. ')
CALL WRTL (1)
CALL FTOM (SINTER,ZZ,4)
CALL WRTR (' 2> Length of Interval = ',ZZ,' m. ')
CALL WRTL (2)
CALL WRTH ('-----')
CALL WRTL (4)

```

C

```

CALL WRITEP
CALL WRTL (5)
DO 50 I=1,NUMSPA
  IF(TYP(I).EQ.3) THEN
CALL WRTH (' ***** ')
WRITE (6,333) I
CALL WRTH (' ***** ')
CALL WRTL(2)
CALL WRTJ(' =====> by now code of beam shape =',TYP(I) )
CALL WRTL(2)
CALL WRTH('      code :      1 = straight haunch          ')
CALL WRTL(1)
CALL WRTH('                  2 = parabolic haunch         ')
CALL WRTL(1)
CALL WRTH('                  3 = constant cross section   ')

```

```

C
CALL WRTL(1)
CALL FTOM(SL(I), ZZ, 4)
CALL WRTR(' 1> span length =', ZZ, 'm. ')
CALL WRTL(1)
  IZZ = NSC(I)
CALL WRTI('          number of section consider =', IZZ, 'sect')
CALL WRTL(1)
CALL FTOM(D1(I), ZZ, 4)
CALL WRTR(' 2> depth of beam =', ZZ, 'm. ')
CALL WRTH('-----')
CALL WRTL(4)
ELSE

```

```

C
C
CALL WRTH(' *****')
WRITE(6, J33) I *****')
333 FORMAT(14X, ' ***** SPAN NUMBER ', IZ, ' *****')
+*)
CALL WRTH(' *****')
CALL WRTL(2)
CALL WRTJ(' ==> by now Code of Beam Shape =', TYP(I) )
CALL WRTL(2)
CALL WRTH(' Code : 1 = Straight haunch ')
CALL WRTL(1)
CALL WRTH(' 2 = Parabolic haunch ')
CALL WRTL(1)
CALL WRTH(' 3 = Constant Cross Section ')
CALL WRTL(1)

```

```

C
CALL WRTL(1)
CALL FTOM(SL(I), ZZ, 4)
CALL WRTR(' 1> Span Length =', ZZ, 'm. ')
CALL WRTL(1)
  IZZ = NSC(I)
CALL WRTI('          Number of Section Consider =', IZZ, 'sect. ')
CALL WRTL(1)
CALL FTOM(D1(I), ZZ, 4)
CALL WRTR(' 2> Left side Depth =', ZZ, 'm. ')
CALL WRTL(1)
CALL FTOM(D0(I), ZZ, 4)
CALL WRTR(' 3> Centre Depth =', ZZ, 'm. ')
CALL WRTL(1)
CALL FTOM(D2(I), ZZ, 4)
CALL WRTR(' 4> Right side Depth =', ZZ, 'm. ')
CALL WRTL(1)
CALL FTOM(SL1(I), ZZ, 4)
CALL WRTR(' 5> Left side Length =', ZZ, 'm. ')
CALL WRTL(1)
CALL FTOM(SL2(I), ZZ, 4)
CALL WRTR(' 6> Right side Length =', ZZ, 'm. ')
CALL WRTL(2)
CALL WRTH('-----')
CALL WRTL(4)
ENDIF
50 CONTINUE
C

```



```
IF (D1(I).GT.DDEPTH) DDEPTH=D1(I)  
IF (D2(I).GT.DDEPTH) DDEPTH=D2(I)  
CONTINUE
```

20  
C

```
NCB = SLM/60 + 1.00  
CBI = SLM/NCB  
LCRSE=(CB/144)*GDSPC/2*145/CBI*SINTER  
LCRSI=(CB/144)*GDSPC*145/CBI*SINTER  
LCURB=(RR/144)*145*SINTER  
IF (LCLASS.EQ.1) THEN  
  ULL = 640*SINTER  
  PLM = 18000  
  PLS = 26000  
ELSE  
  ULL = 480*SINTER  
  PLM = 13500  
  PLS = 19500  
ENDIF
```

C

```
UDLB = BDEPTH*BWIDTH*SINTER*145  
UDLS = GDSPC*(TH/12)*SINTER*(145/2)  
UDPL = (AREA/144)*SINTER*145  
FACM = GDSPC/6  
UDLE = (UDLB+UDLS+(UDLS*2*CEL)/GDSPC+UDPL)/1000  
ADDLE=(LCRSE+LCURB)/1000  
UDLE = UDLE+ADDLE
```

C

```
ULL = (0.5*ULL*FACM)/1000  
PLM = (0.5*PLM*FACM)/1000  
PLS = (0.5*PLS*FACM)/1000  
UDLI = (UDLB+2*UDLS)/1000  
ADDLI=(LCRSI)/1000  
UDLI = UDLI+ADDLI
```

C

```
DI1 = UDLI  
DE1 = UDLE  
ULL = ULL*FAC  
DLL2= ULL  
PLM = PLM*FAC  
PLM = PLM  
PLS = PLS*FAC  
PLS = PLS
```

C

```
RETURN  
END
```

C  
C  
C  
C  
C  
C  
C

-----  
SUDROUTINE MENUCAL  
-----

```
100 CALL CLRSCR  
CALL PAGE (2).  
CALL HEAD ('*****'  
CALL HEAD ('***** CALCULATION MENU *****'  
CALL HEAD ('*****'  
CALL PAGE (2)  
CALL HEAD ('        1> SLAB DESIGN.          '  
CALL PAGE (1)                                     '  
CALL HEAD ('        2> BEAM ANALYSIS.        '  
CALL PAGE (1)
```



```

CALL HEAD ( '          3> BEAM DESIGN.          ' )
CALL PAGE (1)
CALL HEAD ( '          4> <<< RETURN TO MAIN MENU >>>          ' )
CALL PAGE (2)
CALL READI ( '          SELECT NUMBER (1,2,3 or 4) : ' , IWAY )
CALL PAGE (2)
GOTO (1,2,3,4), IWAY
GOTO 100
C
C  SLAB DESIGN PROCESS
1  CALL SLAB
   GOTO 100
C
C  BEAM ANALYSIS FOR VARY CROSS-SECTION
2  CALL ANALYSIS
   GOTO 100
C
C  BEAM DESIGN PROCESS
3  CALL BDESIGN
   GOTO 100
C
C  RETURN TO MAIN MENU
4  RETURN
   END .
C
C
C
C *****
C *                                     *
C *          SUB PROGRAM 2 A          *
C *                                     *
C *          SLAB DESIGN              *
C *                                     *
C *****
C
C
C          |-----|
C          | SUBROUTINE SLAB          |
C          |-----|
C
C
C CHARACTER ANYKEY*15
C CHARACTER HEADI*52
C INTEGER CHANGE, TYP
C
C COMMON /STESEC/ CR, ST(12,2), FORM(12,2)
C
C COMMON /TITLE/ HEADI
C COMMON /MATER/ FCP, FSY
C COMMON /STSIZE/ NOSB, NOST, NO3L
C COMMON /BRGDIA/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
C COMMON /LCLASS/ LCLASS
C COMMON /SETTLE/ STM(10)
C COMMON /PBEAM/ BWIDTH, SINTER
C COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), UO(9), D1(9), D2(9)
C
C COMMON /CDATA/ FC, FS, MR, CK, CJ, CRR, VMAX, VCON1, VCON2, GDSPC
C
C CALL WRITEP
C

```

```

C
55 CALL CLRSCR
   CALL PAGE (1)
C
C
   CALL WRITEP
   WRITE (6,*) ' ***** '
   WRITE (6,*) ' *****          SLAB DESIGN          ***** '
   WRITE (6,*) ' ***** '
C
C
   WRITE (6,*)
   WRITE (6,*)
C DETECT LIVE LOAD SPEC.
   IF (LCLASS.EQ. 1) PLOAD = 16.0
   IF (LCLASS.EQ. 2) PLOAD = 12.0
   WRITE(6,*) ' PLOAD =',PLOAD
C ASSUME GIRDEP WIDTH
   GW = BWIDTH * 12.0
C CALCULATE SLAB EFFECTIVE SPAN LENGTH
   SLSP = GDSPC*12.0 - GW*0.5
   WRITE(6,*) ' SPAN EFF. =',SLSP
C CALCULATE LIVE LOAD BENDING MOMENT
   BMLL = 0.8*(SLSP/12. + 2.0)*PLOAD/32.
   BMLL = BMLL*1000.*12.
C CALCULATE IMPACT BENDING MOMENT
   BMIM = 0.3*BMLL
   WRITE(6,*) ' IMPACT MOMENT =',BMIM
C CALCULATE DEAD WEIGHT OF SLAB
100 DWSL = (TH/12.)*150. + (1.5/12.)*125.
   WRITE(6,*) ' DEAD WEIGHT =',DWSL
C CALCULATE DEAD WEIGHT BENDING MOMENT
   DWBM = DWSL*SLSP*SLSP/(12.*10.)
   WRITE(6,*) ' D.L. MOMENT =',DWBM
C SUMMATION ALL BENDING MOMENT
   TOTM = DWBM + BMLL + BMIM
   WRITE(6,*) ' TOTAL MOMENT =',TOTM
C CALCULATE d FROM ALLOWABLE STRESS DESIGN
   D = SQRT(2.*TOTM/(FC*12.*CJ*CK))
   WRITE(6,*) ' d =',D
C FIND REAL TOTAL DEPTH OF SLAB
   RTH = D + ST(NOSL,1)*0.5 +2.
   RTH = ANINT(2.*RTH + 0.5)
   RTH = RTH/2.
   WRITE(6,*) ' REAL TOTAL DEPTH =',RTH
C CHECK FOR RE-CALCULATION USING TRIAL & ERROR
   IF (RTH.LE. TH) GOTO 200
   TH = RTH
   GOTO 100
C CALCULATE ACTUAL d OF SLAB
200 D = RTH - ST(NOSL,1)*0.5 - 2.
   WRITE(6,*) ' Actual d =',D
C
C
   CALL PAGE (2)
   CALL READC('Press any key : ',ANYKEY)
C
C
C OUTPUT
C

```

```

555 CALL CLRSCR
CALL PAGE (2)
C
WRITE (6,*)
WRITE (6,*)
C CALCULATE AS
AS = TOTN/(FS*CJ*D)
CALL PAGE (2)
CALL PAGE (1)
CALL WRTL (2)
WRITE (6,*)' ===== TRANSVERSE STEEL ====='
CALL WRTL (1)
CALL SPACIN (AS)
C CALCULATE LONGITUDINAL STEEL
AS = 0.67 * AS
NOSL = NOSL - 1
CALL PAGE (2)
CALL PAGE (1)
CALL WRTL (2)
WRITE (6,*)' ===== LONGITUDINAL STEEL ====='
CALL WRTL (1)
CALL SPACIN (AS)
C CALCULATE TEMPERATURE STEEL
AS = 1. / B.
NOSL = 4
CALL PAGE (2)
CALL PAGE (1)
CALL WRTL (2)
WRITE (6,*)' ===== TEMPERATURE STEEL ====='
CALL WRTL (1)
CALL SPACIN (AS)
C
C
CALL PAGE (2)
CALL READC('Press any key : ', ANYKEY)
RETURN
END
C
C
C
C*****
C CALCULATE SPACING OF BAR *
C*****
C
C ----->
C SUBROUTINE SPACIN (AS)
C ----->
C
COMMON /STSIZE/ NOSB, NOST, NOSL
COMMON /STESEC/ CR, ST(12, 2), FORM(12, 2)
C
C CALCULATE SPACING
SP1 = 12. / (AS / ST(NOSL, 2))
SP1 = ANINT(SP1 - 0.5)
IF ( SP1 .GT. 18. ) SP1 = 18.
CALL FTOM (SP1, ZZ, 2)
WRITE(6, 2000) NOSL, ZZ
2000 FORMAT (20X, 'USE #', I2, ' Spacing =', F7.2, ' m. ')
RETURN
END

```





```

COMMON /STSIZE/ NOSB, NOST, NOSL
COMMON /BRGDIA/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
COMMON /LCLASS/ LCLASS
COMMON /SETTLE/ STM(10)
COMMON /PBEAM/ BWIDTH, SINTER
COMMON /DEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), DO(9), D1(9), D2(9)
C
COMMON /BDEPTH/ BDEPTH
COMMON /CDATA/ FC, FS, MR, CK, CJ, CRR, VMAX, VCON1, VCON2, GDSPC
C
COMMON /ZZ/ SK1(9), SK2(9), SK3(9), Q1(9), Q2(9), LL(9)
* . A(9), B(9), C(9), D(9), Z(9, 250), V(9, 250), F(9, 250), DN(100)
COMMON /YY/ AA(100, 100), BB(100, 100), A1(100, 20), B1(100, 20), DF(100)
* . A11(100, 20), B11(100, 20)
COMMON /YY1/ AA1(100, 100), BB1(100, 100)
COMMON /YY3/ Z1(100), A2(100, 12), B2(100, 12), A21(100, 12), B21(100, 12)
COMMON /OVERFLOW/ CCC(100, 100)
COMMON /YY4/ AAA(100, 12), BBB(100, 12)
COMMON /YY5/ AAAA(100, 1), BBBB(100, 1)
COMMON /YY6/ AAAAA(100, 1), BBBBB(100, 1)
COMMON /DIST/ X(9, 50), DX(9, 50)
COMMON /LOAD/DD1, DE1, DI1, DLL2, PLM, PLS
L = NUMSPA
DO 5 I=1,9
LL(I)=NSC(I)-1
5 CONTINUE
C
REWIND 11
REWIND 14
REWIND 18
C
FOR INITIALIZATION OF ALL DATA TO ZERO
C
CALL INITZ(A, B, C, D, Q1, Q2, F, V, Z, L, LL)
CALL DISTAN(L)
CALL PRESTH(L)
C
LR = 7
LC = 8
LD = 11
WRITE(LD, 6000)
WRITE(LD, *) '*****'
WRITE(LD, *) ' ', HEAD1, ' '
WRITE(LD, *) '*****'
WRITE(LD, 901) L
DO 10 I=1, L
WRITE(LD, 902) I, LL(I)
WRITE(LD, 903) SL(I)
10 CONTINUE
901 FORMAT(/, ' NUMBER OF SPANS = ', I4)
902 FORMAT(/, ' SPAN NO. ', I4, ' INTERVAL = ', I4)
903 FORMAT( ' LENGTH = ', F9.4)
C
CALL HADAT(L, LR, LD)
CALL DISFA(L)
L = NUMSPA
LR = 7
LC = 8
LD = 11

```

```

CALL LOAD1 (L,LR,LD)
K = 0
DO 21 I=1,L
K = K + LL(I)
21 CONTINUE
K = K - 1
I1 = 0
DO 31 I = 1,L
DO 41 J = 1,LL(I)
I1 = I1 + 1
41 CONTINUE
31 CONTINUE
CALL ORDIN (AA, A1, K, 1, L, LL, Z1)
CALL ORDIN (BB, B1, K+1, 2, L, LL, Z1)
CALL TRANS
CALL LOADS(L,LR,LD)
C
K=0
DO 20 I=1,L
K = K + LL(I)
20 CONTINUE
K = K - 1
I1 = 0
DO 30 I = 1,L
DO 40 J = 1,LL(I)
I1 = I1+1
C
C IF ( I.EQ.L .AND. J.EQ.LL(I) ) GOTO 40
C READ(LR,99) Z1(I1)
C 99 FORMAT (F10.2)
C
C
40 CONTINUE
30 CONTINUE
C
C CALL ORDIN(AA, A1, K, 1, L, LL, Z1)
C CALL ORDIN(BB, B1, K+1, 2, L, LL, Z1)
C
C
DO 50 JP=1,2
WRITE(LD,6000)
WRITE(LD,*)'*****'
IF (JP.EQ.1) WRITE(LD,*)'EXTERIOR-GIRDER'
IF (JP.EQ.2) WRITE(LD,*)'INTERIOR-GIRDER'
WRITE(LD,*)'*****'
IF (JP.EQ.1) DD1 = DF1
IF (JP.EQ.2) DD1 = DI1
CALL IMPAC(DF, DN, 1, SL, L, LL)
CALL EQUIV(AAA, A1, A11, K, DF, DN, L, LL, LR, LD)
CALL FINSTM(L)
WRITE(LD,1000)
WRITE(LD,2000)
WRITE(LD,5000)(I, (AAA(I, J)=0.1383, J=1, 12), I=1, K)
WRITE(LD,6000)
C
C CALL IMPAC(DF, DN, 2, SL, L, LL)
C CALL EQUIVS(BBB, B1, B11, K+1, DF, DN, L, LL, LR, LD)
C CALL FINSTM(L)
C WRITE(LD,1500)

```

```

WRITE(LD, 2000)
WRITE(LD, 5000) (I, (BBB(I, J)*0.4536, J=1, 12), I=1, K+1)
WRITE(LD, 6000)
IF(JP.EQ.1) THEN
  DO 111 I =1, K
    AAAA(I, 1)=AAA(I, 1)
111 CONTINUE
    DO 333 I =1, K+1
      BBBB(I, 1)=BBB(I, 1)
333 CONTINUE
    ELSE
      DO 555 I =1, K
        AAAAA(I, 1) = AAA(I, 1)
555 CONTINUE
        DO 777 I =1, K+1
          BBBBB(I, 1) = BBB(I, 1)
777 CONTINUE
        ENDIF
50 CONTINUE
C
C
CALL TRUCK (SL, LL, K)
CALL IMPAC (DF, DN, 1, SL, L, LL)
CALL TRUCKT (DF, DN, K, 1)
CALL FINSTM (L)
CALL TRUCKS (SL, LL, K+1)
CALL IMPAC (DF, DN, 2, SL, L, LL)
CALL TRUCTS (DF, DN, K+1)
CALL FINSTH (L)
C
C
DO 60 JP=1, 2
  IF (JP.EQ.1) THEN
    WRITE(LD, 6000)
    WRITE(LD, *) '*****'
    WRITE(LD, *) ' TRUCK EXTERIOR GIRDER '
    WRITE(LD, *) '*****'
C
    WRITE(LD, 3000)
    WRITE(LD, 4000)
    WRITE(LD, 5000) (I, (AA(I, J)*0.1383, J=1, 12), I=1, K)
    WRITE(LD, 6000)
    WRITE(LD, 3500)
    WRITE(LD, 4000)
    WRITE(LD, 5000) (I, (BB(I, J)*0.4536, J=1, 12), I=1, K+1)
    WRITE(LD, 6000)
    ELSE
      WRITE(LD, 6000)
      WRITE(LD, *) '*****'
      WRITE(LD, *) ' TRUCK INTERIOR GIRDER '
      WRITE(LD, *) '*****'
C
    WRITE(LD, 3000)
    WRITE(LD, 4000)
    WRITE(LD, 5000) (I, (AA1(I, J)*0.1383, J=1, 12), I=1, K)
    WRITE(LD, 6000)
    WRITE(LD, 3500)
    WRITE(LD, 4000)
    WRITE(LD, 5000) (I, (BB1(I, J)*0.4536, J=1, 12), I=1, K+1)
    WRITE(LD, 6000)

```





```

COMMON /ZZ/ SK1(9),SK2(9),SK3(9),G1(9),G2(9),LL(9)
* ,A(9),B(9),C(9),D(9),Z(9,250),V(9,250),F(9,250),DN(100)
C
DO 200 I=1,L
CALL HAUNCH(SK1(I),SK2(I),SK3(I),I,FMA,FMB,TYP(I),O.5,SL(I)
* ,SL1(I),SL2(I),D0(I),D1(I),D2(I))
CALL SUMST(I,L)
200 CONTINUE
RETURN
END

C
C
C
C
C FOR SUM STIFFNESS THIS JOINT
C
SUBROUTINE SUMST(I,L)
C
INTEGER TYP
COMMON /BEAM/ TYP(9),SL(9),NSC(9),SL1(9),SL2(9),D0(9),D1(9),D2(9)
COMMON /ZZ/ SK1(9),SK2(9),SK3(9),G1(9),G2(9),LL(9)
* ,A(9),B(9),C(9),D(9),Z(9,250),V(9,250),F(9,250),DN(100)
C
I1 = I+1
D(I) = D(I) + SK1(I)
D(I1) = D(I1)+SK2(I)
C(I) = SK3(I)
A(I1) = SK3(I)
RETURN
END

C
C
C
C
C FOR DISTRIBUTED FACTOR
C
SUBROUTINE DISFA(L)
C
INTEGER TYP
COMMON /BEAM/ TYP(9),SL(9),NSC(9),SL1(9),SL2(9),D0(9),D1(9),D2(9)
COMMON /ZZ/ SK1(9),SK2(9),SK3(9),G1(9),G2(9),LL(9)
* ,A(9),B(9),C(9),D(9),Z(9,250),V(9,250),F(9,250),DN(100)
C
L1 = L+1
DO 100 I=2,L1
I1 = I-1
A(I) = A(I)/D(I1)
D(I) = D(I)-A(I)*C(I1)
100 CONTINUE
RETURN
END

C
C
C
C
C FOR INPUT LOAD
C
SUBROUTINE LOADS(L,LR,LD)
C
INTEGER TYP

```

```

COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), DO(9), D1(9), D2(9)
COMMON /ZZ/ SK1(9), SK2(9), SK3(9), Q1(9), Q2(9), LL(9)
* , A(9), B(9), C(9), D(9), Z(9, 250), V(9, 250), F(9, 250), DMMY(100)
COMMON /YY/ AA(100, 100), BB(100, 100), DMMY1(100, 41)

C
L1 = I+1
L2 = 150
N = 0
DO 100 I=1, L
100 CALL SETPT(SL, Z, I, L, LL)
DO 600 I=1, L
  L3 = LL(I) + 1
  IF (I.EQ.L) L3 = LL(I)
  DO 600 KK=2, L3
    N = N+1
    X = Z(I, KK)
    P = 1.0
    CALL FIXED(SL, X, P, Z, F, V, A1, A2, I, L, LL, TYP, SL1, SL2, DO, D1, D2)
    Q1(I) = Q1(I) + A1
    Q2(I) = Q2(I) + A2
    CALL BALAN(B, A1, A2, I, L, LL)
    CALL CARRY(A, B, C, D, L, LL)
    CALL MEMFC(SL, Z, F, V, B, SK1, SK2, SK3, Q1, Q2, L, LL, LR, LD)
    CALL CHANG(AA, BB, F, V, N, L, LL)
    DO 600 K=1, L1
      B(K) = 0.0
      Q1(K) = 0.0
      Q2(K) = 0.0
      DO 600 J=1, L2
        F(K, J) = 0.0
        V(K, J) = 0.0
600 CONTINUE
DO 200 I=1, N
  BB(N+1, I) = BB(N, I)
  IF (I.EQ.N) BB(N+1, I) = BB(N, I) - 1.
200 CONTINUE
RETURN
END

C
C-----
C FOR INPUT VARY LOAD
C
SUBROUTINE LOAD1 (L, LR, LD)
C
INTEGER TYP
COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), DO(9), D1(9)
* , D2(9)
COMMON /ZZ/ SK1(9), SK2(9), SK3(9), Q1(9), Q2(9), LL(9), A(9), B(9)
* , C(9), D(9), Z(9, 250), V(9, 250), F(9, 250), DMMY(100)
COMMON /YY/ AA(100, 100), BB(100, 100), DMMY1(100, 41)
COMMON /BDEPTH/ BDEPTH
COMMON /DIST/ X(9, 50), DX(9, 50)

C
L1 = L+ 1
L2 = 150
N = 0
DO 100 I = 1, L
100 CALL SETPT (SL, Z, I, L, LL)
DO 600 I = 1, L
  L3 = LL(I) + 1

```

```

      IF(I.EQ.L) L3 = LL(I)
      DO 600 KK =2,L3
      N = N+1
      XX = Z(I, KK)
      P = 1.0*DX(I, KK)/BDEPTH
      CALL FIXED (SL, XX, P, Z, F, V, A1, A2, I, L, LL, TYP, SL1, SL2, DO, D1, D2)
      Q1(I) = Q1(I) + A1
      Q2(I) = Q2(I) + A2
      CALL BALAN (B, A1, A2, I, L, LL)
      CALL CARRY (A, D, C, D, L, LL)
      CALL MENFC (SL, Z, F, V, B, SK1, SK2, SK3, Q1, Q2, L, LL, LR, LD)
      CALL CHANG (AA, BB, F, V, N, L, LL)
      DO 600 K = 1, L1
      B(K) = 0.0
      Q1(K) = 0.0
      Q2(K) = 0.0
      DO 600 J = 1, L2
      F(K, J) = 0.0
      V(K, J) = 0.0
600  CONTINUE
      DO 200 I =1, N
      BB(N+1, I) = BB(N, I)
      IF(I.EQ.N) BB(N+1, I) = BB(N, I)-1
200  CONTINUE
      RETURN
      END
C
C
C   FOR FIXED END POINT LOAD
C
C   SUBROUTINE FIXED(SL, X, P, Z, F, V, A1, A2, I, L, LL, TYP, SL1, SL2, DO, D1, D2)
C
C   INTEGER TYP
C   DIMENSION SL(1), Z(9, 1), F(9, 1), V(9, 1), LL(1)
C   * , TYP(1), SL1(1), SL2(1), DO(1), D1(1), D2(1)
C
C   X2 = X/SL(I)
C   CALL HAUNCH(S1, S2, S3, I, FMA, FMB, TYP(I), X2, SL(I), SL1(I), SL2(I), DO(I)
C   * , D1(I), D2(I))
C   A1 = FMA*SL(I)
C   A2 = -FMB*SL(I)
C   CALL POINT(Z, X, SL(I), P, V, F, I, L, LL)
C   RETURN
C   END
C
C
C
C   FOR BALANCE MOMENT
C
C   SUBROUTINE BALAN(B, A1, A2, I, L, LL)
C
C   DIMENSION B(1), LL(1)
C   I1 = I + 1
C   B(I) = B(I) - A1
C   B(I1) = B(I1) - A2
C   RETURN
C   END
C

```

```

C
C
C FOR CARRY MOMENT
C
C SUBROUTINE CARRY(A, B, C, D, L, LL)
C DIMENSION A(1), B(1), C(1), D(1), LL(1)
C
C L1 = L+1
C DO 100 I = 2, L1
C   I1 = I - 1
100 B(I) = B(I) - A(I)*B(I1)
C   B(L1) = D(L1)/D(L1)
C   DO 200 II=1, L
C     I = L1-II
C     I1 = I+1
200 B(I) = (B(I)-C(I)*B(I1))/D(I)
C   RETURN
C   END
C
C
C FOR FIND MEMBER FORCE
C
C SUBROUTINE MEMFC(SL, Z, F, V, B, SK1, SK2, SK3, G1, G2, L, LL, LR, LD)
C DIMENSION SL(1), Z(9, 1), F(9, 1), V(9, 1), G1(1), G2(1)
C * , SK1(1), SK2(1), SK3(1), B(1), LL(1)
C
C DO 100 I= 1, L
C   I1 = I+1
C   R1 = B(I)
C   R2 = B(I1)
C   S1 = SK1(I)
C   S2 = SK2(I)
C   S3 = SK3(I)
C   P1 = S1*R1+S3*R2
C   P2 = S3*R1+S2*R2
C   P1 = P1 + G1(I)
C   P2 = P2 + G2(I)
C   CALL REACT(SL, Z, P1, P2, V, F, I, L, LL)
100 CONTINUE
C   RETURN
C   END
C
C
C FOR SET DISTANCE THIS POINT
C
C SUBROUTINE SETPT(SL, Z, I, L, LL)
C DIMENSION Z(9, 1), SL(1), LL(1)
C D = SL(I)/LL(I)
C L1 = LL(I)+1
C DO 100 J = 1, L1
100 Z(I, J) = (J-1)*D
C   RETURN
C   END

```

```

C
C
C
C FOR SHEAR AND MOMENT EACH POINT FROM POINT LOAD P
C
C SUBROUTINE POINT(Z, X, SO, P, V, F, I, L, LL)
C
C DIMENSION Z(9, 1), F(9, 1), V(9, 1), LL(1)
C L1 = LL(I) + 1
C J = 0
100 J = J+1
C IF (J.GT.L1) RETURN
C Z1 = Z(I, J)
C IF (Z1.GT.X) GOTO 200
C V2 = P*(SO-X)/SO
C F2 = P*(SO-X)/SO*Z1
300 V(I, J) = V(I, J) + V2
C F(I, J) = F(I, J) + F2
C GOTO 100
200 V2 = -(P*X/SO)
C F2 = P*X/SO*(SO-Z1)
C GOTO 300
C END
C
C
C
C FOR REACTION
C
C SUBROUTINE REACT(SL, Z, P1, P2, V, F, I, L, LL)
C
C DIMENSION SL(1), Z(9, 1), V(9, 1), F(9, 1), LL(1)
C
C L1 = LL(I)+1
C SO = SL(I)
C J = 0
100 J = J+1
C IF (J.GT.L1) RETURN
C V3 = -(P1+P2)/SO
C F3 = P1 + V3*Z(I, J)
C V(I, J) = V(I, J) + V3
C F(I, J) = F(I, J) + F3
C GOTO 100
C END
C
C
C
C FOR CHANGE INFORM INFLUENCE LINE
C
C SUBROUTINE CHANG(AA, BB, F, V, N, L, LL)
C
C DIMENSION AA(100, 1), BB(100, 1), F(9, 1), V(9, 1), LL(1)
C
C NN = 0
C DO 100 I = 1, L
C K = LL(I) + 1
C IF (I.EQ.L) K = LL(I)
C DO 100 J = 2, K

```

```

      NN = NN + 1
      AA(NN,N) = F(I,J)
      BB(NN,N) = V(I,J)
100  CONTINUE
      RETURN
      END

```

```

C
C
C

```

```

C FOR FIND SUM MAX MIN IN MATRIX AA
C

```

```

C SUBROUTINE ORDIN(AA, A1, N, L1, L, LL, Z1)
C

```

```

C DIMENSION AA(100, 1), A1(100, 1), LL(1), H1(9), Z1(1)
C

```

```

      I = 0
      DO 200 I1 = 1, L
      DO 200 I2 = 1, LL(I1)
      IF ((I1, EQ, L, AND, I2, EQ, LL(L)), AND, L1, EQ, 1) GOTO 300
      I = I + 1

```

```

      G = 0.0
      H = 0.0
      H2 = 0.0
      A1(I, 1) = 0.0
      A1(I, 2) = 0.0
      A1(I, 3) = 0.0
      J = 0
      DO 500 J1 = 1, L
      DO 400 J2 = 1, LL(J1)
      J = J + 1

```

```

      IF (J1, EQ, L, AND, J2, EQ, LL(L)) GOTO 400
      A1(I, 3) = AA(I, J) * Z1(J) + A1(I, 3)
      IF (AA(I, J) .GE. 0.0) A1(I, 1) = AA(I, J) + A1(I, 1)
      IF (AA(I, J) .LT. 0.0) A1(I, 2) = AA(I, J) + A1(I, 2)
      IF (AA(I, J) .GE. G .AND. AA(I, J) .GE. 0.0) G = AA(I, J)
      IF (AA(I, J) .LE. H .AND. AA(I, J) .LE. 0.0) H = AA(I, J)

```

```

400  CONTINUE
      IF (L1, EQ, 2 .AND. H2, GT, H) H2 = H
      H1(J1) = H
      H = 0.0

```

```

500  CONTINUE
      A1(I, 3) = A1(I, 1) + A1(I, 2)
      A1(I, 4) = G
      IF (I2, LT, 0.5 * LL(I1) .AND. I1, NE, 1) A1(I, 5) = H1(I1) + H1(I1-1)
      IF (I2, GT, 0.5 * LL(I1) .AND. I1, NE, L) A1(I, 5) = H1(I1) + H1(I1+1)

```

```

200  CONTINUE
300  RETURN
      END

```

```

C
C
C

```

```

C SUBROUTINE TRANS
C

```

```

COMMON/YY/AA(100, 100), BB(100, 100), A1(100, 20), B1(100, 20), BF(100)
* , A11(100, 20), B11(100, 20)
      DO 20 I = 1, 100
      DO 30 J = 1, 20
      A11(I, J) = A1(I, J)
      B11(I, J) = B1(I, J)

```

```

30 CONTINUE
20 CONTINUE
  RETURN
  END
C
C -----
C FOR GIRDER EQUIVALENT - LOADING
C
SUBROUTINE EQUIV (A1, A2, A21, K, DF, DN, L, LL, LR, LD)
COMMON /BEAM/TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D01(9), D02(9)
COMMON /DIST/X(9, 50), DX(9, 50)
COMMON /BDEPTH/ BDEPTH
COMMON /LOAD/DD1, DE1, D11, DLL2, PLM, PLS
COMMON /PBEAM/BWIDTH, SINTER
DIMENSION A1(100, 1), A2(100, 1), A21(100, 1), LL(1), DF(1), DN(1), D(150)
C
DDD1 = (DD1*453.6)/(0.3048*SINTER)
DLLL2 = (DLL2*453.6)/(0.3048*SINTER)
PLMM = PLM*453.6
C
WRITE(LD, *)
WRITE(LD, *) ' UNIFORM DEAD LOAD INTERVAL =', DDD1, 'KO/M'
WRITE(LD, *) ' UNIFORM LIVE LOAD INTERVAL =', DLLL2, 'KO/M'
WRITE(LD, *) ' CONCENTRATED LOAD EQUIVALA =', PLMM, 'KO'
C
C
DO 100 I = 1, K
DO 100 J = 1, 11
  IF (J.EQ.1) A1(I, J) = A21(I, 3)*DD1
  IF (J.EQ.2) A1(I, J) = A2(I, 1)*DLL2
  IF (J.EQ.3) A1(I, J) = A2(I, 2)*DLL2
  IF (J.EQ.4) A1(I, J) = A2(I, 4)*PLM
  IF (J.EQ.5) A1(I, J) = A2(I, 5)*PLM
  IF (J.EQ.6) A1(I, J) = A1(I, 2)+A1(I, 4)
  IF (J.EQ.7) A1(I, J) = A1(I, 3)+A1(I, 5)
  IF (J.EQ.8) A1(I, J) = A1(I, 6)*DF(I)
  IF (J.EQ.9) A1(I, J) = A1(I, 7)*DN(I)
  IF (J.EQ.10) A1(I, J) = A1(I, 1)+A1(I, 6)+A1(I, 8)
  IF (J.EQ.11) A1(I, J) = A1(I, 1)+A1(I, 7)+A1(I, 9)
  IF (A1(I, 10).LT.0.0) A1(I, 10) = 0.0
  IF (A1(I, 11).GT.0.0) A1(I, 11) = 0.0
100 CONTINUE
  RETURN
  END
C -----
C FOR GIRDER EQUIVALENT- LOADING
C
SUBROUTINE EQUIVS (B1, B2, B21, K, DF, DN; L, LL, LR, LD)
COMMON /BEAM/TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D1(9), D2(9)
COMMON /DIST/ X(9, 50), DX(9, 50)
COMMON /LOAD/ DD1, DE1, D11, DLL2, PLM, PLS
COMMON /BDEPTH/BDEPTH
COMMON /PBEAM/ BWIDTH, SINTER
DIMENSION B1(100, 1), D2(100, 1), B21(100, 1), LL(1), DF(1), DN(1)
* , D(150), DD(150)
C
DDD1 = (DD1*453.6)/(SINTER*0.3048)
DLLL2 = (DLL2*453.6)/(SINTER*0.3048)
PLSS = PLS*453.6
C
WRITE(LD, *)

```



```

WRITE(LD,*)' UNIFORM DEAD LOAD INTERVAL=',DD01,'KG/M'
WRITE(LD,*)' UNIFORM LIVE LOAD INTERVAL=',DLL2,'KG/M'
WRITE(LD,*)' CONCENTRATED LOAD INTERVAL=',PLSS,'KG'

```

C

```

DO 100 I=1,K
DO 100 J=1,11
IF(J.EQ.1) B1(I,J) = B21(I,3)*DD1
IF(J.EQ.2) B1(I,J) = B2(I,1)*DLL2
IF(J.EQ.3) B1(I,J) = B2(I,2)*DLL2
IF(J.EQ.4) B1(I,J) = B2(I,4)*PLS
IF(J.EQ.5) B1(I,J) = B2(I,5)*PLS
IF(J.EQ.6) B1(I,J) = B1(I,2)+B1(I,4)
IF(J.EQ.7) B1(I,J) = B1(I,3)+B1(I,5)
IF(J.EQ.8) B1(I,J) = R1(I,6)*DF(I)
IF(J.EQ.9) B1(I,J) = B1(I,7)+DN(I)
IF(J.EQ.10) B1(I,J) = B1(I,1)+B1(I,6)+B1(I,8)
IF(J.EQ.11) B1(I,J) = B1(I,1)+B1(I,7)+B1(I,9)
IF(B1(I,10).LT.0.0) B1(I,10) = 0.0
IF(B1(I,11).GT.0.0) B1(I,11) = 0.0
100 CONTINUE
RETURN
END

```

C

C

C

C FOR COMPUTE IMPACT LOADING

C

C

SUBROUTINE IMPAC(DF, DN, N1, SL, L, LL)

C

DIMENSION LL(1), DF(1), DN(1), SL(1)

C

```

N = 0
DO 100 I=1,L
DO 100 J=1,LL(I)
N=N+1
DF(N) = 50. / (125. + SL(I))
DN(N) = 0.0
IF (J.GT. (LL(I)/2)) DN(N)=50. / (125. +(SL(I)+SL(I+1))/2)
IF (J.LT. (LL(I)/2). AND. I.NE. 1) DN(N)=50. / (125. +(SL(I-1)+SL(I))/2)
IF (J.GT. (LL(I)/2). AND. I.EQ. L) DN(N) = 0.0
IF (N1.EQ. 2) DN(N) = DF(N)
IF (DF(N).GT. 0.3) DF(N) = 0.3
IF (DN(N).GT. 0.3) DN(N) = 0.3
100 CONTINUE
RETURN
END

```

C

C

C

C

C

C FOR OUTPUT DATA

C

SUBROUTINE OUTPUT(A, B, C, N, L, LL, LR, LD)

C

DIMENSION A(100,1), B(100,1), C(7,1), LL(1)

C

```

A(0,10) = 0.0
A(N+1,10) = 0.0
A(0,11) = 0.0
A(N+1,11) = 0.0

```

```

I = -1
I2 = 0
DO 200 I1 = 1,L
  L1 = LL(I1)+1
  DO 100 J1 = 1,L1
    I = I+1
    I2 = I2+1
    B1 = B(I2,10)
    B2 = B(I2,11)
    IF (J1.EQ.L1) THEN
      I2 = I2-1
      IF (I2.EQ.N) THEN
        B((I2+1),10) = 0.0
        B((I2+1),11) = 0.0
      ENDIF
      B1 = B(I2,10) - B((I2+1),11)
      B2 = B((I2+1),10) - B(I2,11)
    ENDIF
    B3 = B1
    IF (ABS(B1).LT.ABS(B2)) B3=ABS(B2)
    WRITE(LD,50) C(I1,J1),A(I,10),A(I,11),B3
50  FORMAT(5F10.2)
    IF (J1.EQ.L1) I=I-1
100  CONTINUE
200  CONTINUE
      RETURN
      END

```

C  
C  
C  
C

---

C SUBROUTINE HAUNCH(KA,KB,KAB,K,FMA,FMB,K1,B,L,L1,L2,D,D1,D2)

C DIMENSION I2(25),S(25),A(25),M1(25),M2(25),M3(25)

C REAL L,L1,L2,I1,I2,KA,KB,KAB

C

X=0.0

F1=0.0

F2=0.0

F3=0.0

C0=0.0

C1=0.0

N=20

M=N+1

IF (K1.EQ.1) CALL STRAI(X,L,L1,L2,D,D1,D2,I2,N,M)

IF (K1.EQ.2) CALL PARAB(X,L,L1,L2,D,D1,D2,I2,N,M)

IF (K1.EQ.3) CALL STRAI(X,L,L1,L2,D,D1,D2,I2,N,M)

I1 = D\*D\*D/12.

DO 100 I=2,N

NM = I/2

A1 = I/2

MM = MM+10

IA1 = A1+10

IF(IA1.EQ.NM) S(I)=4.0

IF(IA1.GT.NM) S(I)=2.0

100 CONTINUE

C

S(1) = 1.0

S(N) = 1.0

X = 0.0

```

DO 200 I=1,M
  M1(I) = (L-X)*(L-X)
  M2(I) = X*X
  M3(I) = X*(L-X)
  X = X + L/N
200 CONTINUE
C
DO 300 I=1,M
  A(I) = S(I)*I1/I2(I)/(3*N)
300 CONTINUE
C
X = 0.0
DO 400 I=1,M
  F1 = F1+M1(I)*A(I)
  F2 = F2+M2(I)*A(I)
  F3 = F3+M3(I)*A(I)
  IF (X.LT.(0.999*B*L)) THEN
    FF1 = F1
    FF2 = F2
    FF3 = F3
  ENDIF
  X = X + 1/N
400 CONTINUE
F4 = FF2*(1-B)
F5 = (F1-FF1)*(B)
F6 = (F3-FF3)*(B)
F7 = FF3*(1-B)
C0 = -(F4+F6)
C1 = -(F5+F7)
FMB = (C0*F1-C1*F3)/(F2*F1-F3*F3)
FMA = (C1*F2-C0*F3)/(F2*F1-F3*F3)
KA = F2/(F1*F2-F3*F3)*L*L
KB = F1/(F1*F2-F3*F3)*L*L
CA = F3/F2
CB = F3/F1
KAB = CA*KA
RETURN
END
C
C
C
SUBROUTINE STRAI(X,L,L1,L2,D,D1,D2,RL,N,M)
C
DIMENSION RL(1)
REAL L,L1,L2
C
DO 100 I=1,M
  DX = D
  IF (X.LT.L1) DX = D+(L1-X)*(D1-D)/L1
  IF (X.GT.(L-L2)) DX = D+(X+L2-L)*(D2-D)/L2
  RL(I) = DX*DX*DX/12.
  X = X+L/N
100 CONTINUE
RETURN
END
C
C
C

```

```

C      SUBROUTINE PARAB(X,L,L1,L2,D,D1,D2,I2,N,M)
C      DIMENSION I2(1)
C      REAL L,L1,L2,I2
C      DO 100 I=1,M
C          DX = D
C          IF (X.LT.L1) DX = D + (L1-X)**2*(D1-D)/L1**2
C          IF (X.GT.(L-L2)) DX = D+(X+L2-L)**2*(D2-D)/L2**2
C          I2(I) = DX*DX*DX/12
C          X = X+L/N
100    CONTINUE
C      RETURN
C      END
C
C
C      SUBROUTINE FIND TRUCK LOAD
C      SUBROUTINE TRUCK(SL,LL,K)
C      DIMENSION SL(1),LL(1)
C      COMMON /PBEAM/ BWIDTH,SINTER
C      COMMON /YY/ B(100,100),XX1(100,100),A(100,20),A1(100,20),XX2(100)
C      COMMON /OVERFLOW/ CCC(100,100)
C
C      R = 4.32/SINTER
C      IR = R
C      F = R-FLD(1R)
C      FACT = F/SINTER
C      DO 500 I2 = 1,K
C          DO 500 J2 = 1,K
C              CCC(I2,J2) = -B(I2,J2)
500    CONTINUE
C      DO 100 I=1,K
C          G = 0.0
C          H = 0.0
C          DO 300 J1 = 1,K
C              IF (B(I,J1).GE.G.AND.B(I,J1).GT.0.0) G = B(I,J1)
C              IF (B(I,J1).LE.H.AND.B(I,J1).LT.0.0) H = B(I,J1)
300    CONTINUE
C      DO 200 J=1,K
C          B(I,0) = 0.0
C          B(I,K+1) = 0.0
C          II = B(I,J)*1000
C          JJ = G*1000
C          KK = H*1000
C          IF (II.EQ.JJ) CALL FINDS(A,B,I,J,IR,1,-FACT,K)
C          IF (II.EQ.KK) THEN
C              CCC(I,0) = 0.0
C              CCC(I,K+1) = 0.0
C              CALL FINDS(A1,CCC,I,J,IR,1,-FACT,K)
C          ENDIF
200    CONTINUE
100    CONTINUE
C      RETURN
C      END
C

```

```

C
C SUBROUTINE FINDS(A, B, I, J, IR, F, K)
C
C DIMENSION A(100, 1), B(100, 1)
C
J1 = J+IR
J2 = J1+1
J3 = J2+1
J4 = J-IR
J5 = J4-1
J6 = J5-1
IF ((J5.GT.0).AND.(J2.LT.K+1)) THEN
  A(I, 1) = B(I, J)
  A(I, 2) = B(I, J5)+F*(B(I, J4)-B(I, J5))
  A(I, 3) = B(I, J2)+F*(B(I, J1)-B(I, J2))
ENDIF
IF ((J5.LE.0).OR.(A(I, 2).LT.0.0)) THEN
  A(I, 1) = B(I, J2)+F*(B(I, J1)-B(I, J2))
  A(I, 2) = B(I, J)
  A(I, 3) = B(I, J3)+F*(B(I, J2)-B(I, J3))
ENDIF
IF ((J2.GE.K+1).OR.(A(I, 3).LT.0.0)) THEN
  A(I, 1) = B(I, J5)+F*(B(I, J4)-B(I, J5))
  A(I, 2) = B(I, J6)+F*(B(I, J5)-B(I, J6))
  A(I, 3) = B(I, J)
ENDIF
IF (A(I, 1).LE.0.0) A(I, 1) = 0.0
IF (A(I, 2).LE.0.0) A(I, 2) = 0.0
IF (A(I, 3).LE.0.0) A(I, 3) = 0.0
IF (A(I, 2).LT.A(I, 3)) THEN
  TT = A(I, 2)
  A(I, 2) = A(I, 3)
  A(I, 3) = TT
ENDIF
RETURN
END
C
C
C
C FOR SET TABLE OF TRUCK LOADING
C
C SUBROUTINE TRUCKT(DF, DN, K)
C
C DIMENSION DF(1), DN(1)
COMMON /YY/ A(100, 100), B(100, 100), A1(100, 20), B1(100, 20), X1(100)
COMMON /YY1/C(100, 100), D(100, 100)
COMMON /YY5/AAAA(100, 1), BBBB(100, 1)
COMMON /YY6/AAAAA(100, 1), BBBBB(100, 1)
C
DO 100 I=1, K
  A(I, 1) = AAAA(I, 1)
  A(I, 2) = A1(I, 1)+A1(I, 2)
  A(I, 3) = -B1(I, 1)-B1(I, 2)
  A(I, 4) = A1(I, 3)
  A(I, 5) = -B1(I, 3)
  A(I, 6) = A(I, 2)*16. +A(I, 4)*4.
  A(I, 7) = A(I, 3)*16. +A(I, 5)*4.
  A(I, 8) = A(I, 6)*DF(I)
  A(I, 9) = A(I, 7)*DN(I)

```

```

A(I,10) = A(I,1)+A(I,6)+A(I,8)
A(I,11) = A(I,1)+A(I,7)+A(I,9)
IF (A(I,10).LT.0.0) A(I,10) = 0.0
IF (A(I,11).GT.0.0) A(I,11) = 0.0
100 CONTINUE
C
DO 200 I=1,K
C(I,1) = AAAAA(I,1)
C(I,2) = A1(I,1)+A1(I,2)
C(I,3) = -B1(I,1)-B1(I,2)
C(I,4) = A1(I,3)
C(I,5) = -B1(I,3)
C(I,6) = C(I,2)*16. +C(I,4)*4.
C(I,7) = C(I,3)*16. +C(I,5)*4.
C(I,8) = C(I,6)*DF(I)
C(I,9) = C(I,7)*DN(I)
C(I,10) = C(I,1)+C(I,6)+C(I,8)
C(I,11) = C(I,1)+C(I,7)+C(I,9)
IF(C(I,10).LT.0.0) C(I,10) = 0.0
IF(C(I,11).GT.0.0) C(I,11) = 0.0
200 CONTINUE
RETURN
END
C
C
C
C
SUBROUTINE FIND TRUCK LOAD
SUBROUTINE TRUCKS(SL,LL,K)
COMMON /PBEAM/ DWIDTH,SINTER
COMMON /YY/ XX1(100,100),B(100,100),A(100,20),A1(100,20),XX2(100)
COMMON /OVERFLOW/ CCC(100,100)
C
R = 4.32/SINTER
IR = R
F = R - FLDAT(IR)
FACT = F/SINTER
DO 500 I2 = 1,K
DO 500 J2 = 1,K
CCC(I2,J2) = -1. * B(I2,J2)
500 CONTINUE
DO 100 I=1,K
G = 0.0
H = 0.0
DO 300 J1=1,K
IF (B(I,J1).GE.G .AND. B(I,J1).GT.0.0) G = B(I,J1)
IF (B(I,J1).LE.H .AND. B(I,J1).LT.0.0) H = B(I,J1)
300 CONTINUE
DO 200 J=1,K
B(I,0) = 0.0
B(I,K+1) = 0.0
II = B(I,J)*1000
JJ = G*1000
KK = H*1000
IF (II.EQ.JJ) CALL FINDS(A,B,I,J,IR,1,-FACT,K)
IF (II.EQ.KK) THEN
CCC(I,0) = 0.0
CCC(I,K+1) = 0.0
CALL FINDS(A1,CCC,I,J,IR,1,-FACT,K)
ENDIF

```

```

200 CONTINUE
100 CONTINUE
RETURN
END

C
C
C
C
C FOR SET TABLE OF TRUCK LOADING
C
C SUBROUTINE TRUCTS(DF, DN, K)
C
C DIMENSION DF(1), DN(1)
COMMON /YY/ B(100, 100), A(100, 100), A1(100, 20), B1(100, 20), X1(100)
COMMON /YY1/C(100, 100), D(100, 100)
COMMON /YY5/ AAAA(100, 1), BBBB(100, 1)
COMMON /YY6/ AAAAA(100, 1), BBBB(100, 1)
C
DO 100 I=1, K
A(I, 1) = BBBB(I, 1)
A(I, 2) = A1(I, 1)+A1(I, 2)
A(I, 3) = -B1(I, 1)-B1(I, 2)
A(I, 4) = A1(I, 3)
A(I, 5) = -B1(I, 3)
A(I, 6) = A(I, 2)*16. +A(I, 4)*4.
A(I, 7) = A(I, 3)*16. +A(I, 5)*4.
A(I, 8) = A(I, 6)*DF(I)
A(I, 9) = A(I, 7)*DN(I)
A(I, 10) = A(I, 1)+A(I, 6)+A(I, 8)
A(I, 11) = -A(I, 1)+A(I, 7)+A(I, 9)
IF (A(I, 10).LT.0.0) A(I, 10) = 0.0
IF (A(I, 11).GT.0.0) A(I, 11) = 0.0
100 CONTINUE
C
DO 200 I=1, K
D(I, 1) = BBBB(I, 1)
D(I, 2) = A1(I, 1)+A1(I, 2)
D(I, 3) = -B1(I, 1)-B1(I, 2)
D(I, 4) = A1(I, 3)
D(I, 5) = -B1(I, 3)
D(I, 6) = D(I, 2)*16. +D(I, 4)*4.
D(I, 7) = D(I, 3)*16. +D(I, 5)*4.
D(I, 8) = D(I, 6)*DF(I)
D(I, 9) = D(I, 7)*DN(I)
D(I, 10) = D(I, 1)+D(I, 6)+D(I, 8)
D(I, 11) = D(I, 1)+D(I, 7)+D(I, 9)
IF(D(I, 10).LT.0.0) D(I, 10) = 0.0
IF(D(I, 11).GT.0.0) D(I, 11) = 0.0
200 CONTINUE
RETURN
END

C
C
C INPUT BEAM GEOMETRY FOR HAUNCH
C
C SUBROUTINE HADAT2(L)
C
C INTEGER TYP
COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D1(9), D2(9)
C

```

```

DO 200 I=1,N
CALL HAUNCH(I, TYP(I), O, B, SL(I), SL1(I), SL2(I), DO(I), D1(I), D2(I))
200 CONTINUE
RETURN
END

C
C
C
C
C
SUBROUTINE HAUNCH2(J, K1, B, L, L1, L2, D, D1, D2)
C
REAL K, KK, KA, KB, KAB
REAL L, L1, L2, I1, I2, KA, KB, KAB
COMMON /STRAI/ S(25), K(9, 2), KK(9, 2), KR(9)
DIMENSION S(25), A(25), M1(25), M2(25), M3(25)
C
X=0.0
F1=0.0
F2=0.0
F3=0.0
C0=0.0
C1=0.0
N=20
M=N+1
IF (K1.EQ.1) CALL STRAI(X, L, L1, L2, D, D1, D2, I2, N, M)
IF (K1.EQ.2) CALL PARAB(X, L, L1, L2, D, D1, D2, I2, N, M)
IF (K1.EQ.3) CALL STRAI(X, L, L1, L2, D, D1, D2, I2, N, M)
I1 = D*D*D
DO 100 I=2, N
MM = I/2
A1 = I/2
MM = MM+1
IA1 = A1
IF (IA1 .EQ. 1) S(I)=4.0
IF (IA1 .EQ. 2) S(I)=2.0
100 CONTINUE
C
S(1) = 1.0
S(N) = 1.0
X = 0.0
DO 200 I=1, N
M1(I) = S(I)*(L-X)
M2(I) = S(I)*L
M3(I) = S(I)*L
X = X + L/N
200 CONTINUE
C
DO 300 I=1, N
A(I) = S(I)*M1(I2(I))/(3*N)
300 CONTINUE
C
X = 0.0
DO 400 I=1, N
F1 = F1 + S(I)*A(I)
F2 = F2 + S(I)*A(I)
F3 = F3 + S(I)*A(I)
IF (X.LT.0.999*B*L) THEN
FF1 = F1
FF2 = F2

```



```

          FF3 = F3
          ENDIF
          X = X + L/N
400 CONTINUE
          F4 = FF2*(1-B)
          F5 = (F1-FF1)*(B)
          F6 = (F3-FF3)*(B)
          F7 = FF3*(1-B)
          C0 = -(F4+F6)
          C1 = -(F5+F7)
          FMB = (C0*F1-C1*F3)/(F2*F1-F3*F3)
          FMA = (C1*F2-C0*F3)/(F2*F1-F3*F3)
          KA = F2/(F1*F2-F3*F3)*L*L
          KB = F1/(F1*F2-F3*F3)*L*L
          CA = F3/F2
          CB = F3/F1
          KAK = CA*KA
C
          COF(J,1)=CA
          COF(J,2)=CB
          KK(J,1)=KA
          KK(J,2)=KB
          KR(J)=KAB
C
          RETURN
          END
C
C
C
C
C
SUBROUTINE DISTAN(L)
C
          INTEGER TYP
          COMMON /PBEAM/ BWIDTH, SINTER
          COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D1(9), D2(9)
          COMMON /DIST/ X(9,50), DX(9,50)
C
C
C Find Lenght along the Span
          WRITE(*,*)
          DO 200 I=1,L
          X(I,1)=0.0
          DO 200 J=2,NSC(I)
          X(I,J)=X(I,J-1)+SINTER
200 CONTINUE
C
C
C Find Depth along the Span
          WRITE(*,*)
          DO 250 I=1,L
          IF (TYP(I).EQ.1) GOTO 300
          IF (TYP(I).EQ.2) GOTO 400
          DO 270 J=1,NSC(I)
          DX(I,J) = D0(I)
270 CONTINUE
          GOTO 250
C
300 WRITE(*,*)
          DO 350 J=1,NSC(I)

```



```

DF(1,1)=1.00
DF(1,2)=K(1,2)/(K(1,2)+K(2,1))
DO 2 I=2,L
DF(I,1)=K(I,1)/(K(I-1,2)+K(I,1))
DF(I,2)=K(I,2)/(K(I,2)+K(I+1,1))
2 CONTINUE
C
C
E=(15120*(FCP*0.0703)**0.5*144)/(0.0703*1000)
DO 3 I=1,L
IC= BWIDTH*DO(I)*DO(I)*DO(I)/12
RR= (STM(I+1)-STM(I))/SL(I)
FEM(I,1)=(KK(I,1)+KR(I))*2*E*IC*RR/SL(I)
FEM(I,2)=(KK(I,2)+KR(I))*2*E*IC*RR/SL(I)
3 CONTINUE
FEM(L+1,1)= 0.0
C
C
C
C
C Find Fixed End Moment
C
BAL(1,1)=-1.0*FEM(1,1)*DF(1,1)
BAL(1,2)=-1.0*(FEM(1,2)+FEM(2,1))*DF(1,2)
DO 5 I=2,L
BAL(I,1)=-1.0*(FEM(I-1,2)+FEM(I,1))*DF(I,1)
BAL(I,2)=-1.0*(FEM(I,2)+FEM(I+1,1))*DF(I,2)
5 CONTINUE
C
DO 7 I=1,L
T(I,1) = FEM(I,1) + BAL(I,1)
T(I,2) = FEM(I,2) + BAL(I,2)
7 CONTINUE
C
C
CO(L+1,1) = 0.0
C
10 DO 40 I=1,L
CO(I,1)=BAL(I,2)*COF(I,2)
CO(I,2)=BAL(I,1)*COF(I,1)
40 CONTINUE
BAL(1,1)=-1.0*CO(1,1)*DF(1,1)
BAL(1,2)=-1.0*(CO(1,2)+CO(2,1))*DF(1,2)
DO 50 I=2,L
BAL(I,1)=-1.0*(CO(I-1,2)+CO(I,1))*DF(I,1)
BAL(I,2)=-1.0*(CO(I,2)+CO(I+1,1))*DF(I,2)
50 CONTINUE
C
DO 60 I=1,L
IF (ABS(CO(I,1)).GT.0.001) GOTO 20
IF (ABS(CO(I,2)).GT.0.001) GOTO 20
60 CONTINUE
GOTO 30
C
20 DO 70 I=1,L
T(I,1) = T(I,1)+BAL(I,1)+CO(I,1)
T(I,2) = T(I,2)+BAL(I,2)+CO(I,2)
70 CONTINUE

```



C find Moment of each segment

C

INTEGER TYP

COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9), D0(9), D1(9), D2(9)

C

COMMON /YY/ AA(100,100), BB(100,100), A1(100,20), B1(100,20), DF(100)

COMMON /YY1/ AA1(100,100), BB1(100,100)

COMMON /YY4/ AA4(100,12), BB4(100,12)

COMMON /STM3/ R(9,2), BM(9,50), V(9,50)

C

C

C

K = 0

DO 10 I=1,L

DO 20 J=1,NSC(I)-1

JK=J+K

AA(JK,10)=BM(I,J+1)\*0.1303

AA1(JK,10)=BM(I,J+1)\*0.1303

AAA(JK,10)=BM(I,J+1)\*0.1303

20 CONTINUE

K = K + NSC(I) - 1

10 CONTINUE

C

C

K=0

DO 15 I=1,L

DO 25 J=1,NSC(I)-1

JK=J+K

AA(JK,11)=AA(JK,1)+AA(JK,6)+AA(JK,8)+AA(JK,10)

AA(JK,12)=AA(JK,1)+AA(JK,7)+AA(JK,9)+AA(JK,10)

IF (AA(JK,11).LT.0.0) AA(JK,11)=0.0

IF (AA(JK,12).GT.0.0) AA(JK,12)=0.0

AA1(JK,11)=AA1(JK,1)+AA1(JK,6)+AA1(JK,8)+AA1(JK,10)

AA1(JK,12)=AA1(JK,1)+AA1(JK,7)+AA1(JK,9)+AA1(JK,10)

IF (AA1(JK,11).LT.0.0) AA1(JK,11)=0.0

IF (AA1(JK,12).GT.0.0) AA1(JK,12)=0.0

AAA(JK,11)=AAA(JK,1)+AAA(JK,6)+AAA(JK,8)+AAA(JK,10)

AAA(JK,12)=AAA(JK,1)+AAA(JK,7)+AAA(JK,9)+AAA(JK,10)

IF (AAA(JK,11).LT.0.0) AAA(JK,11)=0.0

IF (AAA(JK,12).GT.0.0) AAA(JK,12)=0.0

25 CONTINUE

K=K+NSC(I)-1

15 CONTINUE

C

C

C

C

K = 0

DO 30 I=1,L

DO 40 J=1,NSC(I)-1

JK=J+K

BB(JK,10)=(V(I,J)+V(I,J+1))\*0.4536/2

BB1(JK,10)=(V(I,J)+V(I,J+1))\*0.4536/2

BBB(JK,10)=(V(I,J)+V(I,J+1))\*0.4536/2

40 CONTINUE

K=K+NSC(I)-1

30 CONTINUE

C

C

K=0



```

COMMON /IMOMEN/ IEMP(9.50), IEMN(9.50)
COMMON /TMOMEI/ ITMP(9.50), ITMN(9.50)
COMMON /ISHEAR/ IESP(9.50), IESN(9.50)
COMMON /TSHEAI/ ITSP(9.50), ITSN(9.50)
COMMON /MOMENT/ EMP(9.50), EMN(9.50), IMP(9.50), INN(9.50)
COMMON /SFORCE/ ESF(9.50), ISF(9.50)
COMMON /STEELE/ ASTE(9.50), ASBE(9.50), ASDE(9.50)
COMMON /STEELI/ ASTI(9.50), ASBI(9.50), ASDI(9.50)
COMMON /DIST/ X(9.50), DX(9.50)

```

C

```

REWIND 11
REWIND 13

```

C

```

IF (NUMSPA.EQ.1) GOTO 1
IF (NUMSPA.EQ.2) GOTO 2
IF (NUMSPA.EQ.3) GOTO 3
IF (NUMSPA.EQ.4) GOTO 4
IF (NUMSPA.EQ.5) GOTO 5
IF (NUMSPA.EQ.6) GOTO 6
IF (NUMSPA.EQ.7) GOTO 7
IF (NUMSPA.EQ.8) GOTO 8
IF (NUMSPA.EQ.9) GOTO 9

```

```

1 READ(11.801)
  GOTO 10
2 READ(11.802)
  GOTO 10
3 READ(11.803)
  GOTO 10
4 READ(11.804)
  GOTO 10
5 READ(11.805)
  GOTO 10
6 READ(11.806)
  GOTO 10
7 READ(11.807)
  GOTO 10
8 READ(11.808)
  GOTO 10
9 READ(11.809)
  GOTO 10

```

```

801 FORMAT(//)
802 FORMAT(//)
803 FORMAT(//)
804 FORMAT(//)
805 FORMAT(//)
806 FORMAT(//)
807 FORMAT(//)
808 FORMAT(//)
809 FORMAT(//)

```

C

C

C

```

10 WRITE(*,*)
   WRITE(13,*)
   WRITE(13,*)'*****'
   WRITE(13,*)'***** BEAM DESIGN *****'
   WRITE(13,*)'*****'

```

C

C

C Read Exterior Equivalent Moment

```

EEMP(1,1)=0.0
EEMN(1,1)=0.0
C
DO 30 I=1,NUMSPA
DO 30 J=2,NSC(I)
READ(11,999) EEMP(I,J),EEMN(I,J)
EEMP(I,J) = EEMP(I,J)/0.1383
EEMN(I,J) = EEMN(I,J)/0.1383
30 CONTINUE
999 FORMAT(103x,2F10.3)
C
DO 31 I=2,NUMSPA
EEMP(I,1)=EEMP(I-1,NSC(I-1))
EEMN(I,1)=EEMN(I-1,NSC(I-1))
31 CONTINUE
C
DO 32 I=1,NUMSPA
DO 32 J=1,NSC(I)
32 CONTINUE
C
C
C
C Read Exterior Equivalent Shear Force
READ(11,904)
904 FORMAT(/////////)
DO 40 I=1,NUMSPA
DO 40 J=1,NSC(I)-1
READ(11,999) EESP(I,J),EESN(I,J)
EESP(I,J) = EESP(I,J)/0.4536
EESN(I,J) = EESN(I,J)/0.4536
40 CONTINUE
C
C
C
C Read Interior Equivalent Moment
READ(11,905)
905 FORMAT(/////////)
IEMP(1,1)=0.0
IEMN(1,1)=0.0
C
DO 50 I=1,NUMSPA
DO 50 J=2,NSC(I)
READ(11,999) IEMP(I,J),IEMN(I,J)
IEMP(I,J) = IEMP(I,J)/0.1383
IEMN(I,J) = IEMN(I,J)/0.1383
50 CONTINUE
C
DO 51 I=2,NUMSPA
IEMP(I,1)=IEMP(I-1,NSC(I-1))
IEMN(I,1)=IEMN(I-1,NSC(I-1))
51 CONTINUE
C
DO 52 I=1,NUMSPA
DO 52 J=1,NSC(I)
52 CONTINUE
C
C
C
C Read Interior Equivalent Shear Force
READ(11,906)

```



```

906 FORMAT(//////////)
DO 60 I=1,NUMSPA
DO 60 J=1,NSC(I)-1
READ(11,999) IESP(I,J), IESN(I,J)
IESP(I,J) = IESP(I,J)/0.4536
IESN(I,J) = IESN(I,J)/0.4536
60 CONTINUE
C
C
C
C Read Exterior Truck Moment
READ(11,907)
907 FORMAT(//////////)
ETMP(1,1)=0.0
ETMN(1,1)=0.0
C
DO 70 I=1,NUMSPA
DO 70 J=2,NSC(I)
READ(11,999) ETMP(I,J), ETMN(I,J)
ETMP(I,J) = ETMP(I,J)/0.1383
ETMN(I,J) = ETMN(I,J)/0.1383
70 CONTINUE
C
DO 71 I=2,NUMSPA
ETMP(I,1)=ETMP(I-1,NSC(I-1))
ETMN(I,1)=ETMN(I-1,NSC(I-1))
71 CONTINUE
C
DO 72 I=1,NUMSPA
DO 72 J=1,NSC(I)
72 CONTINUE
C
C
C
C Read Exterior Truck Shear Force
READ(11,908)
908 FORMAT(//////////)
C
DO 80 I=1,NUMSPA
DO 80 J=1,NSC(I)-1
READ(11,999) ETSP(I,J), ETSN(I,J)
ETSP(I,J) = ETSP(I,J)/0.4536
ETSN(I,J) = ETSN(I,J)/0.4536
80 CONTINUE
C
C
C read interior truck moment
READ(11,910)
910 FORMAT(//////////)
ITMP(1,1) = 0.0
ITMN(1,1) = 0.0
DO 90 I=1,NUMSPA
DO 90 J=2,NSC(I)
READ(11,999) ITMP(I,J), ITMN(I,J)
ITMP(I,J) = ITMP(I,J)/0.1383
ITMN(I,J) = ITMN(I,J)/0.1383
90 CONTINUE
DO 91 I=2,NUMSPA
ITMP(I,1)=ITMP(I-1,NSC(I-1))
ITMN(I,1)=ITMN(I-1,NSC(I-1))

```

```

91 CONTINUE
DO 92 I=1,NUMSPA
DO 92 J=1,NSC(I)
92 CONTINUE
C
C
C read interior truck shear force
711 READ (11,911)
    FORMAT(////////)
C
DO 100 I=1,NUMSPA
DO 100 J=1,NSC(I)-1
    READ(11,999) ITSP(I,J),ITSN(I,J)
    ITSP(I,J) = ITSP(I,J)/0.4536
    ITSN(I,J) = ITSN(I,J)/0.4536
100 CONTINUE
C
C
C Find Exterior Bending Moment
WRITE(13,*)
WRITE(13,*)
WRITE(13,*) 'EXTERIOR BENDING MOMENT (TON-M)'
DO 101 I=1,NUMSPA
    WRITE(13,*) 'NO. SPAN : ', I
DO 101 J=1,NSC(I)
    EMP(I,J)=EEMP(I,J)
    IF(ETMP(I,J).GT.EEMP(I,J)) EMP(I,J)=ETMP(I,J)
    EMN(I,J)=EEMN(I,J)
    IF(ETMN(I,J).LT.EEMN(I,J)) EMN(I,J)=ETMN(I,J)
    EMP(I,J) = EMP(I,J)*0.1383
    EMN(I,J) = EMN(I,J)*0.1383
    WRITE(13,11) EMP(I,J),EMN(I,J)
11 FORMAT(5X,F10.4,3X,F10.4)
101 CONTINUE
C
C
C Find Exterior Shear Force
WRITE(13,*)
WRITE(13,*)
WRITE(13,*) 'EXTERIOR SHEAR FORCE (TON)'
DO 102 I=1,NUMSPA
    WRITE(13,*) 'NO. SPAN : ', I
DO 102 J=1,NSC(I)-1
    SF1=ABS(EESP(I,J))
    SF2=ABS(EESN(I,J))
    SF3=ABS(ETSP(I,J))
    SF4=ABS(ETSN(I,J))
    ESF(I,J)=SF1
    IF(ESF(I,J).LT.SF2) ESF(I,J)=SF2
    IF(ESF(I,J).LT.SF3) ESF(I,J)=SF3
    IF(ESF(I,J).LT.SF4) ESF(I,J)=SF4
    ESF(I,J) = ESF(I,J)*0.4536
    WRITE(13,12) ESF(I,J)
12 FORMAT(5X,F10.4)
102 CONTINUE
DO 106 I=1,NUMSPA-1
    SE(I+1) =ESF(I,NSC(I)-1)+ESF(I+1,1)
106 CONTINUE
    SE(1) =ESF(1,1)
    SE(NUMSPA+1)=ESF(NUMSPA,NSC(I)-1)
DO 1006 I=1,NUMSPA+1
    REACE(I) =SE(I)*2
1006 CONTINUE

```

```

C
C
C Find interior bending moment
DO 103 I=1,NUMSPA
DO 103J=1,NSC(I)
IMP(I,J) = IEMP(I,J)
IF(ITMP(I,J).GT.IEMP(I,J)) IMP(I,J)=ITMP(I,J)
IMN(I,J) = IEMN(I,J)
IF(ITMN(I,J).LT.IEMN(I,J)) IMN(I,J)=ITMN(I,J)
IMP(I,J) = IMP(I,J)*0.1383
IMN(I,J) = IMN(I,J)*0.1383
103 CONTINUE
C
C Find interior shear force
DO 104 I=1,NUMSPA
DO 104 J=1,NSC(I)-1
SF1 = ABS(IESP(I,J))
SF2 = ABS(IESN(I,J))
SF3 = ABS(ITSP(I,J))
SF4 = ABS(ITSN(I,J))
ISF(I,J)=SF1
IF(ISF(I,J).LT.SF2) ISF(I,J)=SF2
IF(ISF(I,J).LT.SF3) ISF(I,J)=SF3
IF(ISF(I,J).LT.SF4) ISF(I,J)=SF4
ISF(I,J) = ISF(I,J)*0.4536
104 CONTINUE
C
DO 105 I=1,NUMSPA-1
SI(I+1) = ISF(I,NSC(I)-1)+ISF(I+1,1)
105 CONTINUE
SI(1) = ISF(1,1)
SI(NUMSPA+1) = ISF(NUMSPA,NSC(I)-1)
DO 1005 I=1,NUMSPA+1
REAC(I) = SI(I)*(NOBEAM-2)
1005 CONTINUE
C
C
C
DO 470 I=1,NUMSPA
TH =7.0/12
GDSPC=(EGDAP-BWIDTH)/(NOBEAM-1)
B1 = SL(I)/4
B2 = 16*TH + BWIDTH
B3 =0.5*GDSPC + BWIDTH
BB = B1
IF (B2.LT.BB) BB=B2
IF (B3.LT.BB) BB=B3
470 CONTINUE
AA =0.3048* BB
BWIDTH=0.3048*BWIDTH
T=0.3048*TH
CALL EDESIGN(BB)
CALL TDESIGN(BB)
CALL QUANCCN1
CALL QUANCOM2
CALL QUANFORM1
CALL QUANFORM2
CALL QUANSTEEL1
CALL QUANSTEEL2
RETURN
END

```

-----  
SUBROUTINE EDESIGN(BB)

C  
C  
C

```

REAL IMP,IMN,ISF
COMMON /MATER/ FCP,FSY
COMMON /BRGDIA/ NUMTRA,WLANE,EGDAP,NOBEAM,CSL,PAREA,NUMSPA
COMMON /PDEAM/ BWIDTH,SINTER
COMMON /BEAM/ TYP(9),SL(9),NSC(9),SL1(9),SL2(9),D0(9),D1(9),D2(9)
COMMON /STSIZE/ NOSB,NOST,NOSL
COMMON /BDEPTH/ BDEPTH
COMMON /CDATA/ FC,FS,MR,CK,CJ,CRR,VMAX,VCON1,VCON2,QDSPC
COMMON /MOMENT/ EMP(9,50),EMN(9,50),IMP(9,50),INR(9,50)
COMMON /SFORCE/ ESF(9,50),ISF(9,50)
COMMON /DIST/ X(9,50),DX(9,50)
COMMON /STEELE/ ASTE(9,50),ASBE(9,50),ASDE(9,50)
COMMON /STESEC/ CR,ST(12,2),FORM(12,2)

```

C  
C

```

WRITE (13,*)
WRITE (13,*)
WRITE (13,*)' *****'
WRITE (13,*)' EXTERIOR DESIGN '
WRITE (13,*)' *****'

```

C  
C

For Positive Moment

```

DD = 5.0/12
TH = 7.0/12
DO 500 I=1,NUMSPA
DO 500 J=1,NSC(I)
DP = DX(I,J)+TH-DD
AS = EMP(I,J)*1000*12/(FS*(DP*12-TH*12/2))
P = AS/(BB*DP*144)
AK = (MR*P + 0.5*(TH/DP)**2)/(MR*P + TH/DP)
Y = (TH*12*(3*AK*DP*12 - 2*TH*12))/(3*(2*AK*DP*12-TH*12))
AJD = DP*12 - Y
AMC = (1-(TH*12/2*AK*DP*12))*FC*BB*12*TH*12*AJD
ASBE(I,J) = EMP(I,J)*1000*12/(FS*AJD)
500 CONTINUE

```

C  
C

For Negative Moment

```

TH = 7.0/12
DD = 5.0/12
DDD = 5.0/12
DO 55 I=1,NUMSPA
DO 65 J=1,NSC(I)
DN = DX(I,J)+TH-DDD
AM1 = 0.5*FC*CK*CJ*BWIDTH*12*DN*DN*144*(-1)/(1000*12)
IF (AM1.LT.EMN(I,J)) THEN
ASTE(I,J) = EMN(I,J)*(-1)*1000*12/(FS*CJ*DN*12)
ASDE(I,J) = 0.000
ELSE
AS1 = (-1)*AM1*1000*12/(FS*CJ*DN*12)
AM2 = (EMN(I,J)*(-1)*1000*12)-(AM1*(-1)*1000*12)
AS2 = AM2/(FS*(DN*12-DD*12))
ASTE(I,J) = AS1 + AS2
ASDE(I,J) = 0.5*AS2*(1-CK)/(CK-DD/DN)
ENDIF
65 CONTINUE
55 CONTINUE

```

```

C Find bottom steel
  DO 700 I=1,NUMSPA
    DO 800 J=1,NSC(I)
C
  IF(ASDE(I,J).GT.ASBE(I,J)) ASBE(I,J)=ASDE(I,J)
400 CONTINUE
700 CONTINUE
C
C
  VCC = 0.95*FCP**0.5
  ASE = ST(NOST,2)
C
  DO 90 I=1,NUMSPA
    WRITE(13,700)
    WRITE(13,1000) I
    WRITE(13,1500)
    WRITE(13,1100)
    WRITE(13,1110) NOSB,NOSB,NOST
    WRITE(13,1500)
    WRITE(13,*)
  DO 95 J=1,NSC(I)
    NAST = ASDE(I,J)/ST(NOSB,2) + 1
    NASB = ASBE(I,J)/ST(NOSB,2) + 1
    IF(NAST.LT.2) NAST = 2
    IF(NASB.LT.2) NASB = 2
C
    DV = DX(I,J)+TH-DD
    VC = VCC*BWIDTH*DV*144
    V = ESF(I,J)*1000-VC
    IF(V.LT.0) THEN
      S = (DV/2)*0.3048
    ELSE
      S = (2*ASE*FS*DV*12/V)*0.0254
    ENDIF
    IF (S.GT.0.15) S=0.15
    XX = X(I,J)*0.3048
    DDX = DX(I,J)*0.3048
    WRITE(13,1200) J,XX,DDX,NAST,NASB
    IF (J.EQ.NSC(I)) GOTO 95
    WRITE(13,1300) S
  95 CONTINUE
  WRITE(13,*)
  90 CONTINUE
C
C
  900 FORMAT(//)
  1000 FORMAT(1x,'Span # ',I5/)
  1100 FORMAT(' Section Distance Depth Top Bar Bottom Bar
+ Stirrub ')
  1110 FORMAT(' No. (m.) (m.) #',I3,7x,'#',I3,10x,
+' #',I3,' (m.)'//)
  1200 FORMAT(1x,I3,8x,F5.2,5x,F5.2,5x,I6,5x,I6)
  1300 FORMAT(62x,F7.2)
  1500 FORMAT('-----')
+-----')
C
C
C

```



```

IF (AM1.LT. IMN(I,J)) THEN
ASTI(I,J)=IMN(I,J)*(-1)*1000*12/(FS*CJ*DN*12)
ASDI(I,J)=0.000
ELSE
AS1 = (-1)*AM1*1000*12/(FS*CJ*DN*12)
AM2 = IMN(I,J)*(-1)*1000*12-(AM1*(-1)*1000*12)
AS2 = AM2/(FS*(DN*12-DD*12))
ASTI(I,J) = AS1 + AS2
ASDI(I,J) = 0.5*AS2*(1-CK)/(CK-DD/DN)
ENDIF
65 CONTINUE.
:5 CONTINUE
C
C
C Find bottom steel
DO 700 I=1,NUMSPA
DO 900 J=1,NSC(I)
IF(ASDI(I,J).GT. ASBI(I,J)) ASBI(I,J)=ASDI(I,J)
700 CONTINUE
900 CONTINUE
C
C
VCC = 0.95*FCP**0.5
ASI = ST(NOST,2)
C
DO 90 I=1,NUMSPA
WRITE(13,700)
WRITE(13,1000) I
WRITE(13,1500)
WRITE(13,1100)
WRITE(13,1110) NOSB,NOSB,NOST
WRITE(13,1500)
WRITE(13,*)
DO 95 J=1,NSC(I)
NAST = ASTI(I,J)/ST(NOSB,2) + 1
NASB = ASBI(I,J)/ST(NOSB,2) + 1
IF (NAST.LT.2) NAST=2
IF (NASB.LT.2) NASB=2
DV = DX(I,J)+TH-DD
VC = VCC*BWIDTH*DV*144
V = ISF(I,J)*1000-VC
IF (V.LT.0) THEN
S = (DV/2)*0.3048
ELSE
S = (2*ASI*FS*D*12/V)*0.0254
ENDIF
IF(S.GT.0.15) S=0.15
XX = X(I,J)*0.3048
DDX = DX(I,J)*0.3048
WRITE(13,1200) J,XX,DDX,NAST,NASB
IF (J.EQ.NSC(I)) GOTO 95
WRITE(13,1300) S
95 CONTINUE
WRITE(13,*)
90 CONTINUE
C
C
900 FORMAT(//)
1000 FORMAT(1x,'Span # ',I5/)
1100 FORMAT(' Section Distance Depth Top Bar Bottom Bar

```

```

+   Stirrub ' )
1110 FORMAT(' No.      (m.)      (m.)      #' , I3.7x , #' , I3.10x.
+   #' , I3. ' (m.) '//)
1200 FORMAT(1x, I5, 8x, F5. 2, 5x, F5. 2, 5x, I6, 5x, I6)
1300 FORMAT(62x, F7. 2)
1500 FORMAT('-----')
+-----')

```

C  
C  
C  
  
C  
C  
C  
C

RETURN  
END

ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย



```

C-----
C *****
C * QUANTITY OF CONCRETE *
C *****
C-----

```

```

C-----
C SUBROUTINE QUANCON1
C-----

```

```

C * Find Quantity of Concrete Slab *
C

```

```

COMMON /BRGDI/ NUMTRA, WLANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA
COMMON /PBEAM/ BWIDTH, SINTER
COMMON /BEAM/ TYP(9), SL(9), NSC(9), SL1(9), SL2(9)
* DO(9), D1(9), D2(9)
COMMON /DIST/ X(9,50), DX(9,50)

```

```

COMMON /GCON/ TGCS, TGCG, TGC

```

```

TH = 7.00/12
SLL = 0.00
DO 10 I = 1, NUMSPA
SLBL = SL(I) + SLL
CONTINUE
SLBW = EGDAP + CSL*2.00
TGCS = (SLBL*SLBW*TH)*0.3048**3

```

```

C * Find Quantity of Concrete Girder *
C

```

```

GBM = 0.00
DO 20 I = 1, NUMSPA
DO 30 J = 1, NSC(I)-1
N = J+1
GBM = 0.5*(DX(I,J)+DX(I,N))*X(I,N)-X(I,J))*BWIDTH+GBM
30 CONTINUE
20 CONTINUE
GBM = GBM*NOBEAM
TGCG = GBM*0.3048**3
TGC = TGCS+TGCG
RETURN
END

```

```

C-----
C SUBROUTINE QUANCON2
C-----

```

```

C * Quantity of Concrete Slab & Girder
C

```

```

COMMON /GCON/ TGCS, TGCG, TGC

```

```

WRITE(13,600)
WRITE(13,100)
WRITE(13,500)
WRITE(13,200)
WRITE(13,500)
WRITE(13,100)
WRITE(13,300)
WRITE(13,450)
WRITE(13,100)
WRITE(13,400) TGCS, TGCG, TGC
WRITE(13,100)

```

```

600  FORMAT(///// )
500  FORMAT(/)
100  FORMAT(15X, '-----')
    *-----*)
200  FORMAT(T36, 'QUANTITY OF CONCRETE (cu.m)')
300  FORMAT(T20, 'Slab', T40, 'Girder', T60, 'Total')
400  FORMAT(T18, F6.2, T38, F8.2, T62, F(1,1))
450  FORMAT(T20, '(cu.m)', T40, '(cu.m)', T65, '(cu.m)')
      RETURN
      END

C
C-----
C
C *****
C *          QUANTITY OF FORMWORK          *
C *****
C
C-----
C          SUBROUTINE QUANFORM1
C-----
C
C * Find Quantity of Formwork (Slab *
C
C      COMMON /BRGDIA/ NUMTRA, WLANE, I(100), NOBEAM, CSL, PAREA, NUMSPA
C      COMMON /PBEAM/ BWIDTH, SINTER
C      COMMON /BEAM/TYP(9), SL(9), NSC(9), (N)(9), SL2(9), DO(9), D1(9), D2(9)
C      COMMON /DIST/ X(9,50), DX(9,50)
C
C      COMMON /QFORM/ TGFS, TGFG, TGF
C
C      TH = 7.00/12
C      SLBL = 0.00
C      DO 10 I = 1, NUMSPA
C      SLBL = SL(I) + SLBL
C
C      SLBW = EGDAP + CSL*2.00
C      SLBW1 = SLBW-NOBEAM*BWIDTH
C      TGFS = (SLBW1*SLBL+TH*SLBL**2)*0.1048**2
C
C * Find Quantity of Formwork Girder *
C
C      GFMC = 0.00
C      D = 0.00
C      DO 20 I = 1, NUMSPA
C      DO 30 J = 1, NSC(I)-1
C      N = J+1
C      GFMC = 0.5*(DX(I,J)+DX(I,N))*(X(I,N)-X(I,J))+GFMC
C      A = DX(I,J)-DX(I,N)
C      B = X(I,N)-X(I,J)
C      C = A**2+B**2
C      D = C**0.5+D
C
C      CONTINUE
C      CONTINUE
C      GFMC1 = D*BWIDTH+NOBEAM
C      GFMC2 = GFMC+NOBEAM*2
C      TGFG = (GFMC1+GFMC2)*0.304(100)
C      TGF = (TGFG+TGFS)
C      RETURN
C      END
C-----

```

```

C-----
C          SUBROUTINE QUANFORM2
C-----
C * Quantity of Formwork Slab & Girder
C
C          COMMON /QFORM/ TGFS, TGFG, TGF
C
C          WRITE(13,600)
C          WRITE(13,100)
C          WRITE(13,500)
C          WRITE(13,200)
C          WRITE(13,500)
C          WRITE(13,100)
C          WRITE(13,300)
C          WRITE(13,450)
C          WRITE(13,100)
C          WRITE(13,400) TGFS, TGFG, TGF
C          WRITE(13,100)
C          FORMAT(//////)
600      FORMAT(//////)
500      FORMAT(//)
100      FORMAT(15X, '
-----
')
200      FORMAT(T36, 'QUANTITY OF FORMWORK (sq.m)')
300      FORMAT(T20, 'Slab', T40, 'Girder', T40, 'Total')
400      FORMAT(T18, F6.2, T38, F8.2, T62, F11.1)
450      FORMAT(T20, '(sq.m)', T40, '(sq.m)', T65, '(sq.m)')
        RETURN
        END
C-----
C-----
C-----
C * *****
C *          QUANTITY OF STEEL
C * *****
C-----
C          SUBROUTINE QUANSTEEL1
C-----
C * Find Quantity of Steel (slab) *
C
C          COMMON /BRGDIA/ NUMTRA, WLANE, L(10), NOBEAM, CSL, PAREA, NUMSPA
C          COMMON /PBEAM/ BWIDTH, SINTER
C          COMMON /BEAM/ TYP(9), SL(9), NRC(9), SL1(9), SL2(9)
C          * , DO(9), D1(9), D2(9)
C          COMMON /DIST/ X(9,50), DX(9,50)
C
C          *
C          COMMON /NSTEEL/ NASTE(9,50), NASTI(9,50), NASTI(9,50), NASTI(9,50)
C          COMMON /STESEC/ CR, ST(11,2), P(11,2)
C          COMMON /STSIZE/ NOSB, NOST, NOSI
C          COMMON /SPACS/ STRE(9,50), STI(9,50)
C          COMMON /SPACSL/ STT, STL, STTT
C
C          COMMON /GSTEEL/ TGSS, TGSG, TGS
C
C          TH = 7.00/12
C          SLL = 0.00
C          DO 10 I = 1, NUMSPA
C          SLBL = SL(I) + SLL
10      CONTINUE

```

```

SLBW = EGDAP + CSL*2.00
C * Transverse steel
NSTT = SLBL/STT+1
LSTT = NSTT*SLBW
WT = ST(NOSL,2)*5.0812
GST = (LSTT*0.3048)*WT/1000
C * Longitudinal Steel
NOSL = NOSL-1
NSTL = SLBW/STL+1
LSTL = NSTL*SLBL
WT = ST(NOSL,2)*5.0812
QSL = (LSTL*0.3048)*WT/1000
C *Temperature Steel
NSTTT = SLBW/STTT+1
LSTTT = NSTTT*SLBL
WT = ST(NOSL,2)*5.0812
GSTT = (LSTTT*0.3048)*WT/1000
C
TQSS = GST+QSL+GSTT
C
C * Find Quantity of Steel Girder *
C
C * Exterior girder & Interior girder
C * Top Steel & Bottom Steel
C
GSTT2 = 0.00
QSTT22 = 0.00
GSTT4 = 0.00
QSTT44 = 0.00
QSTTB2 = 0.00
QSTTB22 = 0.00
QSTTB4 = 0.00
QSTTB44 = 0.00
DO 20 I = 1,NUMSPA
DO 30 J = 1,NSC(I)
IF(J.EQ.1.OR.J.EQ.NSC(I)) THEN
GSTT1 = NASTE(I,J)*ST(NOSB,?)
QSTTB1 = NASBE(I,J)*ST(NOSB,?)
QSTTT11 = NASTI(I,J)*ST(NOSB,?)
QSTTB11 = NASBI(I,J)*ST(NOSB,?)
QSTTT2 = (QSTTT1*5.0812)*SIN11R/2.00*0.3048+QSTTT2
QSTTB2 = (QSTTB1*5.0812)*SIN11R/2.00*0.3048+QSTTB2
QSTTT22 = (QSTTT11*5.0812)*SIN11R/2.00*0.3048+QSTTT22
QSTTB22 = (QSTTB11*5.0812)*SIN11R/2.00*0.3048+QSTTB22
ELSE
QSTTT3 = NASTE(I,J)*ST(NOSB,?)
QSTTB3 = NASBE(I,J)*ST(NOSB,?)
QSTTT33 = NASTI(I,J)*ST(NOSB,?)
QSTTB33 = NASBI(I,J)*ST(NOSB,?)
QSTTT4 = (QSTTT3*5.0812)*SIN11R*0.3048+QSTTT4
QSTTB4 = (QSTTB3*5.0812)*SIN11R*0.3048+QSTTB4
QSTTT44 = (QSTTT33*5.0812)*SIN11R*0.3048+QSTTT44
QSTTB44 = (QSTTB33*5.0812)*SIN11R*0.3048+QSTTB44
ENDIF
30 CONTINUE
20 CONTINUE
QSG1 = (QSTTT2+QSTTB2+QSTTT4+QSTTB4)*2
QSG2 = (QSTTT22+QSTTB22+QSTTT44+QSTTB44)*(NOBEAM-2)
C
QSG3 = (QSG1+QSG2)/1000

```

```

C
C * Stirrup Steel
C
  NOSTRE = 0
  NOSTRI = 0
  C = 0.00
  DO 40 I = 1, NUMSPA
  DO 50 J = 1, NSC(I)
    N = J+1
    A = ((DX(I, J)+DX(I, N)/7)*PI)*2.00+(4.00/2.54/12))*2
    B = ((BWIDTH-(4.00/2.54/12.00)*2))*2
    C = A+B+C
    NOSTRE = SINTER/(STRE(I, J)*7.0)
    NOSTRI = SINTER/(STRI(I, J)*7.0)
    NOSTRE = NOSTRE+NOSTRE
    NOSTRI = NOSTRI+NOSTRI
50  CONTINUE
40  CONTINUE
  NOSTR = NOSTRE*2+NOSTRI*(NUMSPAN-2)
  GST1 = NOSTR*C*0.3048
C
  GST2 = (ST(NOST, 2)*5.0812)*GST1/1000
C
C
C  TQSG = QSG3+GST2
C
C  TQS = TQSG+TQSS
C
  RETURN
  END
-----
C
C SUBROUTINE QUANSTEEL2
C
C * Quantity of Steel
C
  COMMON /QSTEEL/ TQSS, TQSG, TQS
C
  WRITE(13, 600)
  WRITE(13, 100)
  WRITE(13, 500)
  WRITE(13, 200)
  WRITE(13, 500)
  WRITE(13, 100)
  WRITE(13, 300)
  WRITE(13, 450)
  WRITE(13, 100)
  WRITE(13, 400) TQSS, TQSG, TQS
  WRITE(13, 100)
600  FORMAT(////////)
500  FORMAT(//)
100  FORMAT(15X, '-----
      ')
200  FORMAT(T36, 'QUANTITY OF STEEL (ton)')
300  FORMAT(T20, 'Slab', T40, 'Girder', T60, 'Total')
400  FORMAT(T18, F6.2, T38, F8.2, T63, F10.2)
450  FORMAT(T20, '(ton)', T41, '(ton)', T60, '(ton)')
  RETURN
  END

```

```

C
C-----
SUBROUTINE DEFL1
C-----
C
C
C
C
C
C
C

```

```

INTEGER REACT
COMMON /ADD/ FAC,NCB,E
COMMON/ADD1/SE(10),SI(10),REACE(10),REAC1(10),REACT(10),DEFL(9)
COMMON /ADD2/SM(9),S(9)
COMMON/BEAM/TYP(9),SL(9),NSC(9),SL1(9),SL2(9),DO(9),D1(9),D2(9)
COMMON/PBEAM/BWIDTH,SINTER
COMMON/YY4/ AAA(100,12),BBB(100,12)
COMMON /LOAD/DD1,DE1,DI1,DLL2,PLM,PLS
COMMON/BRGDIA/NUMTRA,WLANE,EGDAP,NOBEAM,CSL,PAREA,NUMSPA,RR,CB

```

```

C
C

```

```

E = E*144
DE1= DE1*1000/SINTER
K1 = NSC(1)-1
K2 = K1+NSC(2)-1
K3 = K2+NSC(3)-1
K4 = K3+NSC(4)-1
K5 = K4+NSC(5)-1
K6 = K5+NSC(6)-1
K7 = K6+NSC(7)-1
K8 = K7+NSC(8)-1
K9 = K8+NSC(9)-1
K = 0
DO 10 I = 1,NUMSPA
K = K+NSC(I)-1
CONTINUE
DO 20 I = 1,K
YI1 = 0.00
IF(I.EQ.K1) YI2 =ABS(AAA(K1,1))*1000
YIII1 = YI2
IF(I.EQ.K2) YII2 =ABS(AAA(K2,1))*1000
YIIII1 = YII2
IF(I.EQ.K3) YIII2 =ABS(AAA(K3,1))*1000
YIY1 = YIII2
IF(I.EQ.K4) YIY2 =ABS(AAA(K4,1))*1000
YY1 = YIY2
IF(I.EQ.K5) YY2 =ABS(AAA(K5,1))*1000
YYI1 = YY2
IF(I.EQ.K6) YYI2 =ABS(AAA(K6,1))*1000
YYIII1 = YYI2
IF(I.EQ.K7) YYII2 =ABS(AAA(K7,1))*1000
YYIIII1 = YYII2
IF(I.EQ.K8) YYIII2 =ABS(AAA(K8,1))*1000
YIX1 = YYIII2
IF(I.EQ.K9) YIX2 =ABS(AAA(K9,1))*1000
CONTINUE
DO 30 I=1,NUMSPA
SM(I) = DE1*SL(I)**2/8
S1= BWIDTH*D1(I)**3/12
S2= BWIDTH*DO(I)**3/12
S3= BWIDTH*D2(I)**3/12
S(I) =(S1+S2+S3)/3
CONTINUE
DO 40 I=1,NUMSPA
IF (I.EQ.NUMSPA) GOTO 50
IF(I.EQ.1)DEFL(I)=SL(I)**2*(5*SM(I)-3*YI2)
+
/(48*E*S(I))

```

```

30

```

C

```

IF(I.EQ.2)DEFL(I)=      **2*(5*SM(I)-3*(YII1+YII2))
+                       -E*S(I)
IF(I.EQ.3)DEFL(I)=      **2*(5*SM(I)-3*(YIII1+YII2))
+                       -E*S(I)
IF(I.EQ.4)DEFL(I)=      **2*(5*SM(I)-3*(YIY1+YY2))
+                       -E*S(I)
IF(I.EQ.5)DEFL(I)=      **2*(5*SM(I)-3*(YY1+YY2))
+                       -E*S(I)
IF(I.EQ.6)DEFL(I)=      **2*(5*SM(I)-3*(YYI1+YYI2))
+                       -E*S(I)
IF(I.EQ.7)DEFL(I)=      **2*(5*SM(I)-3*(YYII1+YYII2))
+                       -E*S(I)
IF(I.EQ.8)DEFL(I)=      **2*(5*SM(I)-3*(YYIII1+YYIII2))
+                       -E*S(I)
IF(I.EQ.9)DEFL(I)=      **2*(5*SM(I)-3*(YIX1+YIX2))
+                       -E*S(I)

```

```

40  CONTINUE
50  DEFL(I) = DEFL(I)
    RETURN
    END

```

C

C

C-----

C

C-----

SUBROUTINE DEFL2

C-----

C

```

INTEGER REACT
COMMON /ADD/ FAC, REACT(10), REACE(10), REACI(10), REACT(10), DEFL(9)
COMMON /ADD1/ SE(10)
COMMON /ADD2/ SM(7)
COMMON /BEAM/ TYP(7), NSC(7), SL1(9), SL2(9), DO(9), D1(9), D2(9)
COMMON /PBEAM/ BWIDT, P, W, H, L, B, T, S, R, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z
COMMON /YY4/ AAA(10), BBB(100, 12)
COMMON /LOAD/ DDI, DDD, DDD1, DLL2, PLM, PLS
COMMON /BRODIA/ NUMT, LANE, EGDAP, NOBEAM, CSL, PAREA, NUMSPA, RR, CB

```

C

C

```

WRITE(13, 600)
WRITE(13, 100)
WRITE(13, 500)
WRITE(13, 300)
WRITE(13, 500)
WRITE(13, 100)
WRITE(13, 500)
DO 1 I=1, NUMSPA
WRITE(13, 1000) DEFL(I)*12*2.54
CONTINUE
WRITE(13, 500)
WRITE(13, 100)
600  FORMAT(//////)
100  FORMAT(15X, '-----')
500  FORMAT(/)
300  FORMAT(T20, 'DEFLECTION EACH SPAN (MIDDLE SPAN)', '(CM. )')
1000  FORMAT(T16, 'DEF. EACH SPAN ', I1, 2X, '= ', F10.3, ' CM. ')

```

```

WRITE(13, 600)
WRITE(13, 100)
WRITE(13, 500)

```

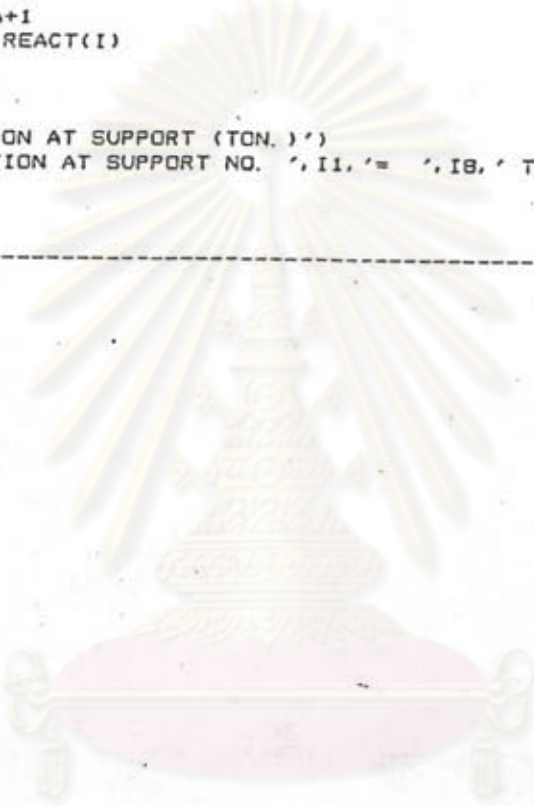
C

```
WRITE(13,301)
WRITE(13,500)
WRITE(13,100)
WRITE(13,500)
DO 20 I=1,NUMSPA+1
WRITE(13,1001) I,REACT(I)
20 CONTINUE
WRITE(13,500)
WRITE(13,100)
301 FORMAT(T20,'REACTION AT SUPPORT (TON. )')
1001 FORMAT(T16,'REACTION AT SUPPORT NO. ',I1,' = ',I8,' TON. ')
RETURN
END
```

C

C

C



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จุฬาลงกรณ์มหาวิทยาลัย





ภาคผนวก ข.  
การใช้โปรแกรมคอมพิวเตอร์

ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย

## การใช้โปรแกรมคอมพิวเตอร์

ในการใช้โปรแกรมคอมพิวเตอร์ของงานวิจัยนี้ วิธีการใช้ง่ายและสะดวก การใช้จะเป็นการเลือกหัวข้อซึ่งเป็นหมายเลข ผู้ใช้โปรแกรมจะเป็นผู้เลือกหัวข้อโดยการพิมพ์ตัวเลขหัวข้อหลังจากนั้นกด RETURN KEY บนแป้นพิมพ์ ผู้ใช้ก็สามารถดำเนินการอย่างใดอย่างหนึ่งได้ตามหัวข้อที่ตนเองได้เลือกไว้

เมื่อเข้าสู่ตัวโปรแกรมเริ่มต้นจะขึ้นด้วยรายการหลัก (Main Menu) ให้ผู้ใช้ทำการเลือกโดยมีหัวข้อใหญ่ 3 หัวข้อคือ

- 1) EDIT DATA MENU
- 2) PROCESS CALCULATION :  
Then output are on file OUT 1. TXT  
and OUT 2. TXT  
and OUT 3. TXT
- 3) QUIT TO PRIME OPERATING SYSTEM  
SELECT NUMBER (1, 2, 3)

เมื่อผู้ใช้โปรแกรมเลือกหัวข้อ 1 ซึ่งเป็นหัวข้อของการใส่ข้อมูล จะขึ้นรายละเอียดในหัวข้อที่ 1 ให้เลือกสำหรับการใส่ข้อมูลอีก 7 หัวข้อคือ

- 1) Edit Title Heading
- 2) Edit Material Properties
- 3) Edit Load Class
- 4) Edit Bridge Dimensions
- 5) Edit Beam Dimensions
- 6) Edit Bar Size
- 7) Edit Settlement Size
- 8) <<< Return to main menu >>>

SELECT NUMBER (1,2,.....,8)

ผู้ใช้โปรแกรมจะเป็นผู้เลือกการใส่ข้อมูลที่ละหัวข้อ หลังจากใส่ข้อมูลครบทุกหัวข้อแล้วก็ให้ทำการเลือกหัวข้อที่ 8 ก็จะกลับคืนสู่รายการหลัก (Main Menu) เริ่มแรกอีกครั้งหนึ่ง หลัง

จากนี้ให้ผู้ใช้เลือกหัวข้อที่ 2 ในรายการหลัก (Main Menu) ซึ่งคือส่วนการวิเคราะห์และการออกแบบ ก็จะมีรายละเอียดให้เลือกอีก 3 หัวข้อคือ

- 1) SLAB DESIGN
- 2) BEAM ANALYSIS
- 3) BEAM DESIGN
- 4) <<<RETURN TO MAIN MENU>>>

SELECT NUMBER (1, 2, 3, 4)

หลังจากผู้ใช้โปรแกรมได้ทำการเลือกหัวข้อ 1, 2, 3 ซึ่งเป็นการวิเคราะห์และการออกแบบเสร็จเรียบร้อยแล้ว และต้องการกลับสู่ รายการหลัก (Main menu) ก็ให้เลือกหัวข้อที่ 4 สำหรับข้อมูลที่ได้ทำการป้อนในครั้งแรกจะถูกเก็บไว้ในไฟล์ชื่อ OUT 1.TXT ส่วนผลของการวิเคราะห์และการออกแบบจะเก็บไว้ในไฟล์ชื่อ OUT 2. TXT และ OUT 3. TXT และเมื่อผู้ใช้โปรแกรมเลือกหัวข้อ 3 ในรายการหลัก (Main Menu) ก็จะเป็นการสิ้นสุดของการใช้โปรแกรม

ในการออกแบบทุกครั้งผู้ใช้โปรแกรมสามารถจะทำการเปลี่ยนแปลงข้อมูลได้ทุกครั้งโดยการป้อนข้อมูลตัวใหม่ได้

ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย

ภาคผนวก ค.

การคำนวณจากโปรแกรมคอมพิวเตอร์



ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย

\*\*\*\*\*  
\*\*\*\*\*

WWWW	WWW	WWWWW	WWWW	WWW	W	WWWWW	WWW	WWW	WWW
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWWW	W	WWWWW	WWWWW	WWW	W	W	W	WWW	WWW
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWWWW	WWW	WWWWW	WWWWW	WWW	WWW	WWW	WWW	WWW	WWW

WWW	W	W	WWWWW	W	WWWWW	W	W	WWWWW
W	W	W	W	W	WW	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
WWW	WWW	W	WWW	WW	W	W	W	W

\*\*\*\*\*  
\*\*\*\*\*

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\*\*\*\*\*  
\*\*\*\*\* TITLE HEADING LIST \*\*\*\*\*  
\*\*\*\*\*

An Existing Title is shown below

CONTINUOUS R.C. DEAM FOR BRIDGE DESIGN

---

\*\*\*\*\*  
\*\*\*\*\* MATERIAL PROPERTIES \*\*\*\*\*  
\*\*\*\*\*

- 1> Ultimate Strength of concrete 180.00 ksc.
  - 2> Yield Strength of steel 2500.00 ksc.
- 

\*\*\*\*\*  
\*\*\*\*\* LOAD CLASS LISTING \*\*\*\*\*  
\*\*\*\*\*

====> by now Load Class is HS20

---

\*\*\*\*\*  
 \*\*\*\*\* BRIDGE DIMENSION LISTING \*\*\*\*\*  
 \*\*\*\*\*

1> Number of Traffic Lane	2
2> Lane Width	3.00 m.
3> Outer Dimension Appart Ex-Gir.	8.00 m.
4> Number of Girder per section	4
5> Overhanging Slab from Girder	0.30 m.
6> Area of Parapet Family	0.20 sq. m.
7> Number of span	3
8> Curb area	0.10 sq. m.
9> Cross beam area	0.10 sq. m.

---

\*\*\*\*\*  
 \*\*\*\*\* BEAM DIMENSION LISTING \*\*\*\*\*  
 \*\*\*\*\*

1> Beam Width	=	0.30 m.
2> Length of Interval	=	5.03 m.

---

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\*\*\*\*\*  
\*\*\*\*\* SPAN NUMBER 1 \*\*\*\*\*  
\*\*\*\*\*

====> by now Code of Beam Shape = 1

Code :        1 = Straight haunch  
              2 = Parabolic haunch  
              3 = Constant Cross Section

1> Span Length                =     30.18 m.  
    Number of Section Consider =     7 sect.  
2> Left side Depth            =     2.00 m.  
3> Centre      Depth           =     1.80 m.  
4> Right side Depth           =     2.00 m.  
5> Left side Length           =     6.71 m.  
6> Right side Length          =     6.71 m.

---

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\*\*\*\*\*  
\*\*\*\*\* SPAN NUMBER 2 \*\*\*\*\*  
\*\*\*\*\*

====> by now Code of Beam Shape = 1

Code :        1 = Straight haunch  
              2 = Parabolic haunch  
              3 = Constant Cross Section

1> Span Length            =    40.23 m.  
    Number of Section Consider =        9 sect.  
2> Left side Depth        =    2.00 m.  
3> Centre     Depth        =    1.00 m.  
4> Right side Depth       =    2.00 m.  
5> Left side Length       =    6.71 m.  
6> Right side Length      =    6.71 m.

---

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\*\*\*\*\*  
\*\*\*\*\* SPAN NUMBER 3 \*\*\*\*\*  
\*\*\*\*\*

====> by now Code of Beam Shape = 1

Code :        1 = Straight haunch  
              2 = Parabolic haunch  
              3 = Constant Cross Section

1> Span Length                =        30.10 m.  
    Number of Section Consider =        7 sect.  
2> Left side Depth            =        2.00 m.  
3> Centre        Depth        =        1.80 m.  
4> Right side Depth          =        2.00 m.  
5> Left side Length          =        6.71 m.  
6> Right side Length         =        6.71 m.

---

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\*\*\*\*\*  
 \*\*\*\*\* BAR SIZE LISTING \*\*\*\*\*  
 \*\*\*\*\*

Bar #	1	2	3	4	5	6	7	8	9	10	11	12
Diam. (mm.)	4	6	9	12	15	19	22	25	28	30	33	36
1> BEAM Bar Size							#		8			
2> BEAM Stirrup Bar Size							#		3			
3> SLAB Bar Size							#		4			

---

\*\*\*\*\*  
 \*\*\*\*\* SETTLEMENT LISTING \*\*\*\*\*  
 \*\*\*\*\*

1> Settlement at Joint # 1 = 0.00 m.  
 2> Settlement at Joint # 2 = 0.01 m.  
 3> Settlement at Joint # 3 = 0.01 m.  
 4> Settlement at Joint # 4 = 0.00 m.

---

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WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW
W	W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W	W
WWW	W	WWW	WWW	WWW	W	W	W	WWW	WWW	W
W	W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W	W
WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW

WWW	W	W	WWW	WWW	WWW	W	W	WWW
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
WWW	WWW	W	WWW	WWW	W	W	W	W

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ศูนย์วิทยทรัพยากร  
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CONTINUOUS R. C. BEAM FOR BRIDGE DESIGN  
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NUMBER OF SPANS = 3  
SPAN NO. 1 INTERVAL = 6  
LENGTH = 99.0000  
SPAN NO. 2 INTERVAL = 6  
LENGTH = 132.0000  
SPAN NO. 3 INTERVAL = 6  
LENGTH = 99.0000

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EXTERIOR-CIRDER  
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UNIFORM LIVE LOAD INTERVAL = 668.367 KG/M  
CONCENTRATED LOAD EQUIVALA = 5729.58 KG

GIRDER BENDING MOMENT-EQUIVALENT LOADING (TON-M)

	DEAD-LOAD	LIVE-LOAD	CONCEN.-LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL
1	88.8743	37.0338 -9.5416	22.6487 0.0000	59.6825 -9.5416	13.3220 0.0000	-0.1231	161.7557 0.0000
2	119.0609	57.1576 -19.0832	33.3998 0.0000	90.5574 -19.0832	20.2137 0.0000	-0.2462	229.5858 0.0000
3	92.1459	60.3711 -28.6248	33.6754 0.0000	94.0465 -28.6248	20.9925 0.0000	-0.3693	206.8156 0.0000
4	8.1292	46.6746 -38.1664	25.9484 -13.3030	72.6230 -51.4695	16.2105 -10.7005	-0.4924	96.4702 -54.5333
5	-132.9892	19.9095 -51.5495	13.5127 -19.6015	33.4221 -71.1510	7.4603 -14.7923	-0.6156	0.0000 -219.5481
6	-332.7956	11.8360 -100.5345	5.2287 -39.0761	17.0647 -139.6106	3.8091 -29.0251	-0.7387	0.0000 -502.1700
7	-131.3506	16.2966 -45.8097	14.6905 -20.8601	30.9872 -66.6698	6.0286 -13.8607	-0.7387	0.0000 -212.6197
8	11.4036	44.2113 -31.4490	27.2737 -13.0340	71.4850 -44.4830	13.9076 -9.2480	-0.7387	96.0575 -43.0661
9	97.0561	69.5766 -31.4491	35.9605 -9.9902	105.5371 -41.4393	20.5325 -8.6152	-0.7387	222.3870 0.0000
10	125.6069	78.0317 -31.4491	38.9586 0.0000	116.9902 -31.4491	22.7607 0.0000	-0.7387	264.6192 0.0000
11	97.0561	69.5766 -31.4492	35.9605 -9.9903	105.5371 -41.4395	20.5325 -8.6153	-0.7387	222.3870 0.0000
12	11.4036	44.2115 -31.4492	27.2739 -13.0341	71.4854 -44.4834	13.9077 -9.2481	-0.7387	96.0580 -43.0666
13	-131.3507	16.2970 -45.8101	14.6908 -20.8603	30.9878 -66.6704	6.0287 -13.8608	-0.7387	0.0000 -212.6206
14	-332.7961	11.8360 -100.5346	5.2287 -39.0765	17.0647 -139.6111	3.8200 -29.0252	-0.7387	0.0000 -502.1712
15	-132.9899	19.9093 -51.5495	13.5125 -19.6018	33.4218 -71.1513	7.4602 -14.7924	-0.6156	0.0000 -219.5492
16	8.1287	46.6743 -38.1662	25.9483 -13.3032	72.6225 -51.4693	16.2104 -10.7005	-0.4925	96.4692 -54.5335
17	92.1457	60.3709 -28.6246	33.6753 0.0000	94.0462 -28.6246	20.9924 0.0000	-0.3693	206.8149 0.0000
18	119.0609	57.1574 -19.0831	33.3998 0.0000	90.5572 -19.0831	20.2136 0.0000	-0.2462	229.5855 0.0000
19	88.8743	37.0338 -9.5415	22.6487 0.0000	59.6825 -9.5415	13.3220 0.0000	-0.1231	161.7558 0.0000

UNIFORM LIVE LOAD INTERVAL= 668.367 KG/M  
 CONCENTRATED LOAD INTERVAL= 8276.06 KG

GIRDER SHEAR-EQUIVALENT LOADING (TON)

	DEAD-LOAD	LIVE-LOAD	CONCEN.	-LOAD	TOTAL LL.	IMPACT	LOAD	SETTLE	TOTAL
1	17.2342	7.3615	6.5030	0.0000	13.8645	3.0947	-0.4234	0.0803	34.2737
2	5.8537	4.7203	4.7949	0.0000	9.5152	2.1239	-0.5841	0.0803	17.5731
3	-5.2193	2.7728	3.2230	0.0000	5.9958	1.3383	-0.8997	0.0803	2.1951
4	-16.2922	1.4638	1.8626	-6.0080	3.3264	0.7425	-2.6989	0.0803	0.0000
5	-27.3652	0.7073	0.7760	-7.3684	1.4832	0.3311	-3.5840	0.0803	0.0000
6	-38.7457	0.3921	0.2502	-8.4550	0.6423	0.1434	-4.5065	0.0803	0.0000
7	39.0635	13.1343	7.4818	0.0000	20.6161	4.0109	-0.2665	0.0000	63.6905
8	27.6823	10.0956	6.4937	-0.7942	16.5892	3.2275	-0.4837	0.0000	47.4990
9	16.6094	7.4581	5.3515	-1.7824	12.8096	2.4921	-0.8168	0.0000	31.9112
10	5.5365	5.2846	4.1380	0.0000	9.4227	1.8332	-0.7012	0.0000	16.7923
11	-5.5365	3.6039	2.9245	-5.0120	6.5285	1.2701	-2.0032	0.0000	2.2621
12	-16.6094	2.4161	1.7824	-6.2255	4.1985	0.8168	-2.6622	0.0000	0.0000
13	-27.6824	1.6922	0.8739	-7.3676	2.5661	0.4992	-3.3975	0.0000	0.0000
14	-39.0636	1.3696	0.8739	-8.3558	2.2435	0.4365	-4.1810	0.0000	0.0000
15	38.7457	11.7340	7.5001	0.0000	19.2341	4.2933	-0.0875	-0.0803	62.1929
16	27.3652	8.6878	6.4135	-0.7760	15.1013	3.3708	-0.3311	-0.0803	45.7570
17	16.2922	6.0830	5.0531	0.0000	11.1361	2.4857	-0.3267	-0.0803	29.8338
18	5.2193	4.0307	3.4811	0.0000	7.5118	1.6767	-0.6189	-0.0803	14.3276
19	-5.8537	2.6168	1.7731	0.0000	4.3899	0.9799	-1.0536	-0.0803	0.0000
20	-18.1570	1.8966	0.9549	0.0000	2.8516	0.6365	-1.6432	-0.0803	0.0000
									-27.2419

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 INTERIOR-GIRDER  
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UNIFORM LIVE LOAD INTERVAL = 668.367 KG/M  
 CONCENTRATED LOAD EQUIVALENT = 5729.56 KG

GIRDER BENDING MOMENT-EQUIVALENT LOADING (TON-M)

	DEAD-LOAD	LIVE-LOAD	CONCEN. -LOAD	TOTAL LL	IMPACT LOAD	SETTLE	TOTAL					
1	86.7009	37.0338	-9.5416	22.6487	0.0000	59.6825	-9.5416	13.3220	0.0000	-0.1231	159.5823	0.0000
2	116.1493	57.1576	-19.0832	33.3996	0.0000	90.5574	-19.0832	20.2137	0.0000	-0.2462	226.6741	0.0000
3	89.8925	60.3711	-28.6248	33.6754	0.0000	94.0465	-28.6248	20.9925	0.0000	-0.3693	204.5622	0.0000
4	7.9304	46.6746	-38.1664	25.9484	-13.3030	72.6230	-31.4695	16.2105	-10.7005	-0.4924	96.2715	-54.7321
5	-129.7369	19.9095	-51.5495	13.5127	-19.6015	33.4221	-71.1510	7.4603	-14.7923	-0.6156	0.0000	-216.2959
6	-324.6571	11.8360	-100.5345	5.2287	-39.0761	17.0647	-139.6106	3.8091	-29.0251	-0.7387	0.0000	-494.0315
7	-128.1384	16.2968	-45.8097	14.6905	-20.8601	30.9872	-66.6698	6.0286	-13.8607	-0.7387	0.0000	-209.4275
8	11.1247	44.2113	-31.4490	27.2737	-13.0340	71.4850	-44.4830	13.9076	-9.2480	-0.7387	95.7786	-43.3444
9	94.6826	69.5766	-31.4491	35.9605	-9.9902	105.5371	-41.4393	20.5325	-8.6152	-0.7387	220.0135	0.0000
10	122.5352	78.0317	-31.4491	35.9586	0.0000	116.9902	-31.4491	22.7607	0.0000	-0.7387	261.5475	0.0000
11	94.6826	69.5766	-31.4492	35.9605	-9.9903	105.5371	-41.4395	20.5325	-8.6153	-0.7387	220.0135	0.0000
12	11.1247	44.2115	-31.4492	27.2739	-13.0341	71.4854	-44.4834	13.9077	-9.2481	-0.7387	95.7791	-43.3455
13	-128.1385	16.2970	-45.8101	14.6908	-20.8603	30.9878	-66.6704	6.0287	-13.8608	-0.7387	0.0000	-209.4084
14	-324.6577	11.8360	-100.5346	5.2287	-39.0765	17.0647	-139.6111	3.8200	-29.0252	-0.7387	0.0000	-494.0327
15	-129.7376	19.9093	-51.5495	13.5125	-19.6018	33.4218	-71.1513	7.4602	-14.7924	-0.6156	0.0000	-216.2969
16	7.9299	46.6743	-38.1662	25.9483	-13.3032	72.6225	-31.4693	16.2104	-10.7005	-0.4925	96.2704	-54.7323
17	89.8923	60.3709	-28.6246	33.6753	0.0000	94.0462	-28.6246	20.9924	0.0000	-0.3693	204.5615	0.0000
18	116.1493	57.1574	-19.0831	33.3998	0.0000	90.5572	-19.0831	20.2136	0.0000	-0.2462	226.6739	0.0000
19	86.7011	37.0338	-9.5415	22.6487	0.0000	59.6825	-9.5415	13.3220	0.0000	-0.1231	159.5824	0.0000

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UNIFORM LIVE LOAD INTERVAL= 665.367 KC/M  
 CONCENTRATED LOAD INTERVAL= 5276.06 KG

GIRDER SHEAR-EQUIVALENT LOADING (TON)

	DEAD-LOAD	LIVE-LOAD	CONCEN. -LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL
1	17.6662	7.3615 -1.8967	6.5030 0.0000	13.8645 -1.8967	3.0947 -0.4234	0.0803	34.7057 0.0000
2	6.0004	4.7203 -2.6168	4.7949 0.0000	9.5152 -2.6168	2.1239 -0.5841	0.0803	17.7198 0.0000
3	-5.3501	2.7728 -4.0307	3.2230 0.0000	5.9958 -4.0307	1.3383 -0.8997	0.0803	2.0643 -10.2007
4	-16.7006	1.4638 -6.0830	1.8626 -6.0080	3.3264 -12.0910	0.7425 -2.6989	0.0803	0.0000 -31.4102
5	-28.0512	0.7073 -8.6879	0.7760 -7.3684	1.4832 -16.0962	0.3311 -3.5840	0.0803	0.0000 -47.6111
6	-39.7170	0.3921 -11.7341	0.2502 -8.4550	0.6423 -20.1891	0.1434 -4.5065	0.0803	0.0000 -64.3323
7	40.0427	13.1343 -1.3696	7.4818 0.0000	20.6161 -1.3696	4.0109 -0.2665	0.0000	64.6695 0.0000
8	28.3763	10.0956 -1.6922	6.4937 -0.7942	16.5892 -2.4864	3.2275 -0.4837	0.0000	48.1930 0.0000
9	17.0258	7.4581 -2.4161	5.3515 -1.7824	12.8096 -4.1985	2.4921 -0.8168	0.0000	32.3276 0.0000
10	5.6752	5.2846 -3.6039	4.1380 0.0000	9.4227 -3.6039	1.8332 -0.7012	0.0000	16.9311 0.0000
11	-5.6753	3.6039 -5.2846	2.9245 -5.0120	6.5285 -10.2966	1.2701 -2.0032	0.0000	2.1233 -17.9751
12	-17.0258	2.4161 -7.4582	1.7824 -6.2255	4.1985 -13.6836	0.8168 -2.6622	0.0000	0.0000 -33.3716
13	-28.3763	1.6922 -10.0956	0.8739 -7.3676	2.5661 -17.4632	0.4992 -3.3975	0.0000	0.0000 -49.2370
14	-40.0428	1.3696 -13.1343	0.8739 -8.3558	2.2435 -21.4901	0.4365 -4.1810	0.0000	0.0000 -65.7139
15	39.7170	11.7340 -0.3921	7.5001 0.0000	19.2341 -0.3921	4.2933 -0.0875	-0.0803	63.1642 0.0000
16	26.0512	8.6878 -0.7073	6.4135 -0.7760	15.1013 -1.4832	3.3708 -0.3311	-0.0803	46.4430 0.0000
17	16.7006	6.0830 -1.4638	5.0531 0.0000	11.1361 -1.4638	2.4857 -0.3267	-0.0803	30.2422 0.0000
18	5.3501	4.0307 -2.7728	3.4811 0.0000	7.5116 -2.7728	1.6767 -0.6189	-0.0803	14.4584 0.0000
19	-6.0004	2.6168 -4.7203	1.7731 0.0000	4.3899 -4.7203	0.9799 -1.0536	-0.0803	0.0000 -11.8546
20	-18.6122	1.8966 -7.3615	0.9549 0.0000	2.8516 -7.3615	0.6365 -1.6432	-0.0803	0.0000 -27.6971

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 TRUCK EXTERIOR GIRDER  
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GIRDER BENDING MOMENT-TRUCK LOADING (TON-M)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	88.8743	3.5786	-0.5262	1.3152	-0.2611	62.5191	-9.4630	13.9552	0.0000	-0.1231	165.2255	0.0000
2	119.0609	5.2746	-1.0523	2.6229	-0.5221	94.8861	-18.9260	21.1799	0.0000	-0.2462	234.8807	0.0000
3	92.1459	5.3165	-1.5782	2.6482	-0.7813	95.6560	-28.3759	21.3518	0.0000	-0.3693	208.7843	0.0000
4	8.1292	4.0962	-3.6134	2.0353	-1.7903	73.6798	-64.9765	16.4464	-13.5086	-0.4924	97.7630	-70.8484
5	-132.9892	2.1271	-5.6487	1.0528	-2.7994	38.2449	-101.5771	8.5368	-21.1179	-0.6156	0.0000	-256.2999
6	-332.7956	0.8278	-7.6840	0.4126	-3.8084	14.8943	-138.1778	3.3246	-28.7272	-0.7387	0.0000	-500.4393
7	-131.3506	2.3134	-2.5453	1.1446	-1.2686	41.5933	-45.7987	8.0921	-9.5216	-0.7387	0.0000	-187.4099
8	11.4036	4.3059	-2.0634	2.1399	-1.0284	77.4543	-37.1283	15.0689	-7.7190	-0.7387	103.1862	-34.1623
9	97.0561	5.6751	-1.5815	2.7900	-0.7883	101.9625	-28.4578	19.8371	-5.9164	-0.7387	218.1169	0.0000
10	125.6069	4.0674	-1.4625	1.9964	-0.6946	73.0638	-26.1776	14.2147	0.0000	-0.7387	212.1468	0.0000
11	97.0561	2.4596	-1.5816	1.2029	-0.7883	44.1651	-26.4563	8.5924	-5.9165	-0.7387	149.0749	0.0000
12	11.4036	1.8731	-2.0634	0.9247	-1.0284	33.6686	-37.1288	6.9503	-7.7191	-0.7387	50.8838	-34.1830
13	-131.3507	2.3076	-2.5453	1.1256	-1.2686	41.4246	-45.7993	8.0593	-9.5217	-0.7387	0.0000	-187.4104
14	-332.7961	0.8278	-4.2117	0.4126	-2.0819	14.8943	-75.7156	2.8977	-15.7413	-0.7387	0.0000	-424.9918
15	-132.9899	2.1271	-3.5098	1.0528	-1.7350	38.2449	-63.0964	8.5367	-13.1178	-0.6156	0.0000	-209.8196
16	8.1287	4.0961	-2.8078	2.0353	-1.3680	73.6794	-50.4771	16.4463	-10.4942	-0.4925	97.7619	-53.3351
17	92.1457	5.3164	-2.1059	2.6482	-1.0410	95.6556	-37.8576	21.3517	0.0000	-0.3693	208.7837	0.0000
18	119.0609	5.2746	-1.4039	2.6229	-0.6940	94.8859	-25.2386	21.1799	0.0000	-0.2462	234.8805	0.0000
19	88.8743	3.5786	-0.7020	1.3152	-0.3470	62.5191	-12.6193	13.9552	0.0000	-0.1231	165.2256	0.0000

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GIRDER SHEAR-TRUCK LOADING (TON)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL
1	17.6662	0.7743 -0.1046	0.3834 -0.0502	13.9216 -1.8744	3.1075 -0.4164	0.0803	34.7756 0.0000
2	6.0004	0.5242 -0.1968	0.2571 -0.0975	9.4162 -3.5381	2.1018 -0.7898	0.0803	17.5987 0.0000
3	-5.3501	0.3521 -0.3801	0.1708 -0.1850	6.3171 -6.8216	1.4101 -1.5227	0.0803	2.4573 -13.6141
4	-16.7006	0.2032 -0.5525	0.0961 -0.2709	3.6359 -9.9243	0.8116 -2.2153	0.0803	0.0000 -28.7600
5	-28.0512	0.1593 -0.7018	0.0762 -0.3453	2.8607 -12.6105	0.6385 -2.8149	0.0803	0.0000 -43.3963
6	-39.7170	0.1593 -0.8212	0.0782 -0.4045	2.8607 -14.7573	0.6385 -3.2940	0.0803	0.0000 -57.6881
7	40.0427	0.8193 -0.0958	0.4026 -0.0477	14.7227 -1.7235	2.8643 -0.3353	0.0000	57.6297 0.0000
8	28.3763	0.7471 -0.0958	0.3705 -0.0477	13.4353 -1.7235	2.6139 -0.3353	0.0000	44.4254 0.0000
9	17.0258	0.5820 -0.1945	0.2871 -0.0915	10.4598 -3.4783	2.0350 -0.6767	0.0000	29.5206 0.0000
10	5.6752	0.0958 -0.3635	0.0477 -0.1796	1.7235 -6.5346	0.3353 -1.2713	0.0000	7.7340 -2.1307
11	-5.6753	0.0958 -0.3635	0.0477 -0.1796	1.7235 -6.5346	0.3353 -1.2713	0.0000	0.0000 -13.4613
12	-17.0258	0.0958 -0.3635	0.0477 -0.1796	1.7235 -6.5346	0.3353 -1.2713	0.0000	0.0000 -24.6318
13	-28.3763	0.0958 -0.3635	0.0477 -0.1796	1.7235 -6.5346	0.3353 -1.2713	0.0000	0.0000 -36.1623
14	-40.0428	0.0958 -0.6165	0.0477 -0.4036	1.7235 -14.6788	0.3353 -2.8558	0.0000	0.0000 -57.5774
15	39.7170	0.8212 -0.1593	0.4045 -0.0784	14.7573 -2.8616	3.2940 -0.6387	-0.0803	57.6881 0.0000
16	28.0512	0.7018 -0.1593	0.3453 -0.0784	12.6106 -2.8616	2.8149 -0.6387	-0.0803	43.3963 0.0000
17	16.7006	0.5525 -0.2032	0.2709 -0.0961	9.9244 -3.6359	2.2153 -0.8116	-0.0803	28.7600 0.0000
18	5.3501	0.3801 -0.3521	0.1850 -0.1708	6.8216 -6.3171	1.5227 -1.4101	-0.0803	13.6141 -2.4572
19	-6.0004	0.1928 -0.5242	0.0915 -0.2571	3.4510 -9.4162	0.7703 -2.1018	-0.0803	0.0000 -17.5987
20	-18.6122	0.0783 -0.7114	0.0374 -0.3508	1.4024 -12.7847	0.3130 -2.8537	-0.0803	0.0000 -34.3308

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\*\*\*\*\*  
 TRUCK INTERIOR GIRDER  
 \*\*\*\*\*

GIRDER BENDING MOMENT-TRUCK LOADING (TON-M)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	86.7009	3.5786	-0.5262	1.3152	-0.2611	62.5191	-9.4630	13.9552	0.0000	-0.1231	163.0521	0.0000
2	116.1493	5.2746	-1.0523	2.6229	-0.5221	94.8861	-18.9260	21.1799	0.0000	-0.2462	231.9691	0.0000
3	89.8925	5.3165	-1.5782	2.6482	-0.7813	95.6560	-28.3759	21.3518	0.0000	-0.3693	206.5309	0.0000
4	7.9304	4.0962	-3.6134	2.0353	-1.7903	73.6798	-64.9765	16.4464	-13.5086	-0.4924	97.5642	-71.0472
5	-129.7369	2.1271	-5.6487	1.0528	-2.7994	38.2449	-101.5771	8.5368	-21.1179	-0.6156	0.0000	-253.0476
6	-324.6571	0.8278	-7.6840	0.4126	-3.8084	14.8943	-138.1778	3.3246	-26.7272	-0.7387	0.0000	-492.3008
7	-128.1384	2.3134	-2.5453	1.1446	-1.2686	41.5933	-45.7987	8.0921	-9.5216	-0.7387	0.0000	-184.1974
8	11.1247	4.3059	-2.0634	2.1399	-1.0284	77.4543	-37.1283	15.0689	-7.7190	-0.7387	102.9093	-34.4612
9	94.6826	5.6751	-1.5815	2.7900	-0.7883	101.9625	-28.4578	19.8371	-5.9164	-0.7387	215.7434	0.0000
10	122.5352	4.0674	-1.4625	1.9964	-0.6946	73.0638	-26.1776	14.2147	0.0000	-0.7387	209.0750	0.0000
11	94.6826	2.4596	-1.5816	1.2029	-0.7683	44.1651	-28.4583	8.5924	-5.9165	-0.7387	146.7014	0.0000
12	11.1247	1.8731	-2.0634	0.9247	-1.0284	33.6686	-37.1288	6.5503	-7.7191	-0.7387	50.6049	-34.4619
13	-128.1385	2.3076	-2.5453	1.1256	-1.2686	41.4246	-45.7993	8.0593	-9.5217	-0.7387	0.0000	-184.1982
14	-324.6577	0.8278	-4.2117	0.4126	-2.0819	14.8943	-75.7156	2.8977	-15.7413	-0.7387	0.0000	-416.8533
15	-129.7376	2.1271	-3.5098	1.0528	-1.7350	38.2445	-63.0964	8.5367	-13.1178	-0.6156	0.0000	-206.5674
16	7.9299	4.0961	-2.8078	2.0353	-1.3880	73.6794	-50.4771	16.4463	-10.4942	-0.4925	97.5631	-53.9338
17	89.8923	5.3164	-2.1059	2.6482	-1.0410	95.6556	-37.8578	21.3517	0.0000	-0.3693	206.5302	0.0000
18	116.1493	5.2746	-1.4039	2.6229	-0.6940	94.8859	-25.2386	21.1799	0.0000	-0.2462	231.9689	0.0000
19	86.7011	3.5786	-0.7020	1.3152	-0.3470	62.5191	-12.6193	13.9552	0.0000	-0.1231	163.0522	0.0000

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GIRDER SHEAR-TRUCK LOADING (TON)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	17.2342	0.7743	-0.1046	0.3834	-0.0502	13.9216	-1.8744	3.1075	-0.4184	0.0803	34.3436	0.0000
2	5.8537	0.5242	-0.1968	0.2571	-0.0975	9.4162	-3.5381	2.1018	-0.7898	0.0803	17.4520	0.0000
3	-5.2193	0.3521	-0.3801	0.1708	-0.1850	6.3171	-6.8216	1.4101	-1.5227	0.0803	2.5881	-13.4833
4	-16.2922	0.2032	-0.5525	0.0961	-0.2709	3.6359	-9.9243	0.8116	-2.2153	0.0803	0.0000	-26.3516
5	-27.3652	0.1593	-0.7018	0.0782	-0.3453	2.8607	-12.6105	0.6385	-2.8149	0.0803	0.0000	-42.7103
6	-38.7457	0.1593	-0.8212	0.0782	-0.4045	2.8607	-14.7573	0.6385	-3.2940	0.0803	0.0000	-56.7169
7	39.0635	0.8193	-0.0958	0.4036	-0.0477	14.7227	-1.7235	2.8643	-0.3353	0.0000	56.6505	0.0000
8	27.6823	0.7471	-0.0958	0.3705	-0.0477	13.4353	-1.7235	2.6139	-0.3353	0.0000	43.7315	0.0000
9	16.6094	0.5820	-0.1945	0.2871	-0.0915	10.4598	-3.4783	2.0350	-0.6767	0.0000	29.1042	0.0000
10	5.5365	0.0958	-0.3635	0.0477	-0.1796	1.7235	-6.5346	0.3353	-1.2713	0.0000	7.5952	-2.2695
11	-5.5365	0.0958	-0.3635	0.0477	-0.1796	1.7235	-6.5346	0.3353	-1.2713	0.0000	0.0000	-13.3425
12	-16.6094	0.0958	-0.3635	0.0477	-0.1796	1.7235	-6.5346	0.3353	-1.2713	0.0000	0.0000	-24.4154
13	-27.6824	0.0958	-0.3635	0.0477	-0.1796	1.7235	-6.5346	0.3353	-1.2713	0.0000	0.0000	-35.4884
14	-39.0636	0.0958	-0.8165	0.0477	-0.4036	1.7235	-14.6786	0.3353	-2.8558	0.0000	0.0000	-56.5982
15	38.7457	0.8212	-0.1593	0.4045	-0.0784	14.7573	-2.8616	3.2940	-0.6387	-0.0803	56.7168	0.0000
16	27.3652	0.7018	-0.1593	0.3453	-0.0784	12.6106	-2.8616	2.8149	-0.6387	-0.0803	42.7103	0.0000
17	16.2922	0.5525	-0.2032	0.2709	-0.0961	9.9244	-3.6359	2.2153	-0.8116	-0.0803	25.3516	0.0000
18	5.2193	0.3801	-0.3521	0.1850	-0.1708	6.8216	-6.3171	1.5227	-1.4101	-0.0803	13.4833	-2.5881
19	-5.8537	0.1928	-0.5242	0.0915	-0.2571	3.4510	-9.4162	0.7703	-2.1018	-0.0803	0.0000	-17.4520
20	-18.1570	0.0783	-0.7114	0.0374	-0.3508	1.4024	-12.7847	0.3130	-2.8537	-0.0803	0.0000	-33.6757

\*\*\*\*\* END OF BEAM ANALYSIS \*\*\*\*\*

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WWWW WWW WWWWWW WWW WWW W WWWWWW WWW WWW WWW  
W  
W  
WWW W WWWWWW WWW WWW W W W W W W W W W W W W  
W  
WWWWWW WWW WWWWWW WWWWWW WWW WWW WWW WWW WWW WWW

WWW W W WWWWWW WWW WWWWWW W W WWWWWW  
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WWW WWW W WWWWWW WWW W W W W W W W W W W

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\*\*\*\*\*  
 \*\*\*\*\* SLAB DESIGN \*\*\*\*\*  
 \*\*\*\*\*

PLDAD = 16.0000  
 SPAN eff. = 95.1450  
 IMPACT MOMENT = 14297.4  
 DEAD WEIGHT = 15.6250  
 D.L. MOMENT = 1178.72  
 TOTAL MOMENT = 63134.1  
 d = 6.15334  
 REAL TOTAL DEPTH = 8.50000  
 DEAD WEIGHT = 121.875  
 D.L. MOMENT = 9174.01  
 TOTAL MOMENT = 71149.4  
 d = 6.53228  
 REAL TOTAL DEPTH = 9.00000  
 DEAD WEIGHT = 128.125  
 D.L. MOMENT = 9655.50  
 TOTAL MOMENT = 71620.9  
 d = 6.55389  
 REAL TOTAL DEPTH = 9.00000  
 Actual d = 6.75000

\*\*\*\*\* TRANSVERSE STEEL \*\*\*\*\*

USE # 4 Spacing = 0.10 m.

\*\*\*\*\* LONGITUDINAL STEEL \*\*\*\*\*

USE # 3 Spacing = 0.08 m.

\*\*\*\*\* TEMPERATURE STEEL \*\*\*\*\*

USE # 4 Spacing = 0.46 m.

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\*\*\*\*\*  
 \*\*\*\*\* BEAM DESIGN \*\*\*\*\*  
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\*\*\*\*\*  
 EXTERIOR DESIGN  
 \*\*\*\*\*

Span # 1

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	2	2	0.40
2	5.03	1.85	2	5	0.91
3	10.06	1.80	2	7	0.89
4	15.09	1.80	2	7	0.39
5	20.12	1.80	2	3	0.18
6	25.15	1.85	9	7	0.12
7	30.18	2.00	16	16	

Span # 2

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	16	16	0.13
2	5.03	1.85	7	5	0.19
3	10.06	1.80	2	3	0.37
4	15.09	1.80	2	7	0.89
5	20.12	1.80	2	8	15.97
6	25.15	1.80	2	7	0.35
7	30.18	1.80	2	3	0.17
8	35.21	1.85	7	5	

9      40.24      2.00      16      16      0.12

Span #      3

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	16	16	0.14
2	5.03	1.85	7	5	0.20
3	10.06	1.80	2	3	0.43
4	15.09	1.80	2	7	0.89
5	20.12	1.80	2	7	847.55
6	25.15	1.85	2	5	0.34
7	30.18	2.00	2	2	

\*\*\*\*\*  
INTERIOR DESIGN  
\*\*\*\*\*

Span #      1

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	2	2	0.45
2	5.03	1.85	2	5	0.91
3	10.06	1.80	2	7	0.89
4	15.09	1.80	2	6	0.44
5	20.12	1.80	2	3	0.20
6	25.15	1.85	8	6	0.13
7	30.18	2.00	10	15	



Span # 2

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	15	15	
2	5.03	1.85	7	4	0.15
3	10.06	1.80	2	3	0.20
4	15.09	1.80	2	7	0.41
5	20.12	1.80	2	8	0.87
6	25.15	1.80	2	7	0.87
7	30.18	1.80	2	3	0.38
8	35.21	1.85	7	4	0.17
9	40.24	2.00	15	15	0.13

Span # 3

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	15	15	
2	5.03	1.85	7	5	0.15
3	10.06	1.80	2	3	0.22
4	15.09	1.80	2	6	0.47
5	20.12	1.80	2	7	0.87
6	25.15	1.85	2	5	0.87
7	30.18	2.00	2	2	0.38

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 QUANTITY OF CONCRETE (cu. m)
 

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	Girder (cu. m)	Total (cu. m)
110.01	222.74	376.55

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 QUANTITY OF FORMWORK (sq. m)
 

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	Girder (sq. m)	Total (sq. m)
75.01	1605.63	2385.75

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 QUANTITY OF STEEL (ton)
 

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	Girder (ton)	Total (ton)
43.77	43.77	45.69

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## DEFLECTION EACH SPAN (MIDDLE SPAN) (CM. )

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DEF. FOR SPAN 1	=	2.831	CM.
DEF. FOR SPAN 2	=	6.974	CM.
DEF. FOR SPAN 3	=	2.831	CM.

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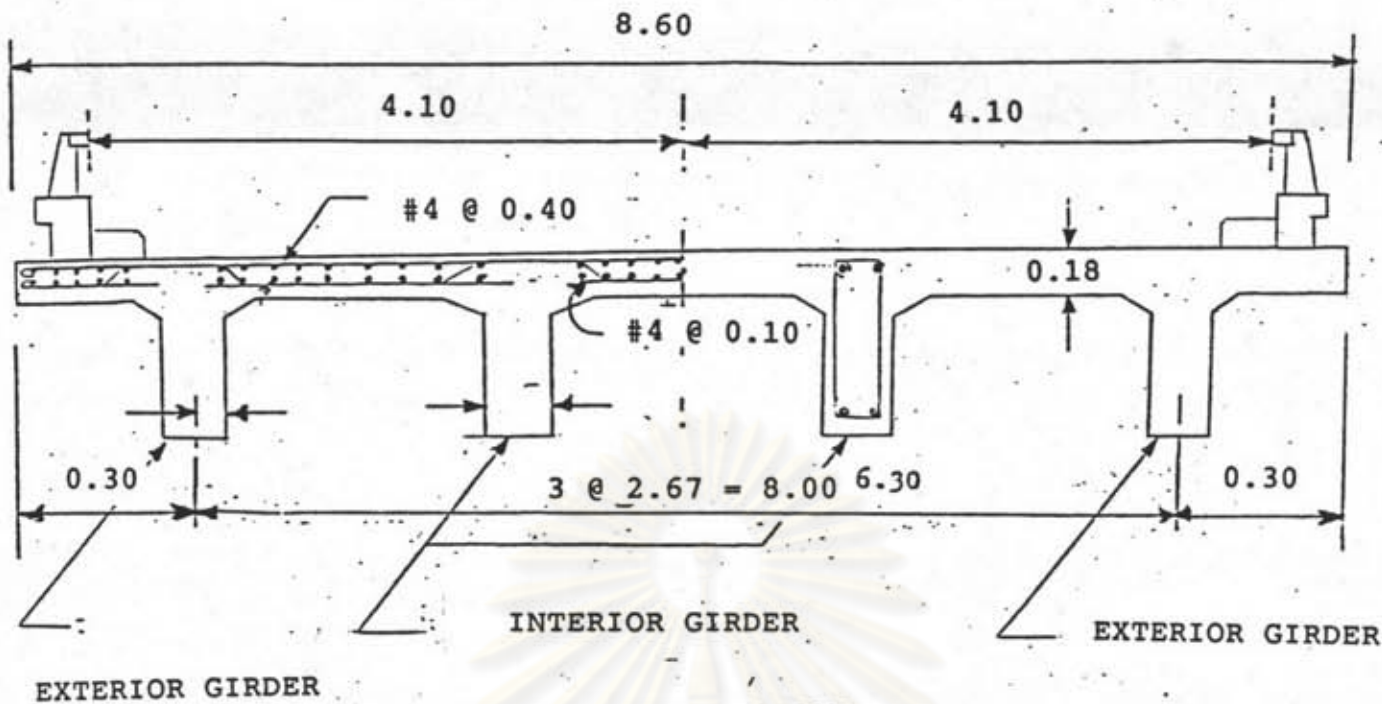
## REACTION AT SUPPORT (TON. )

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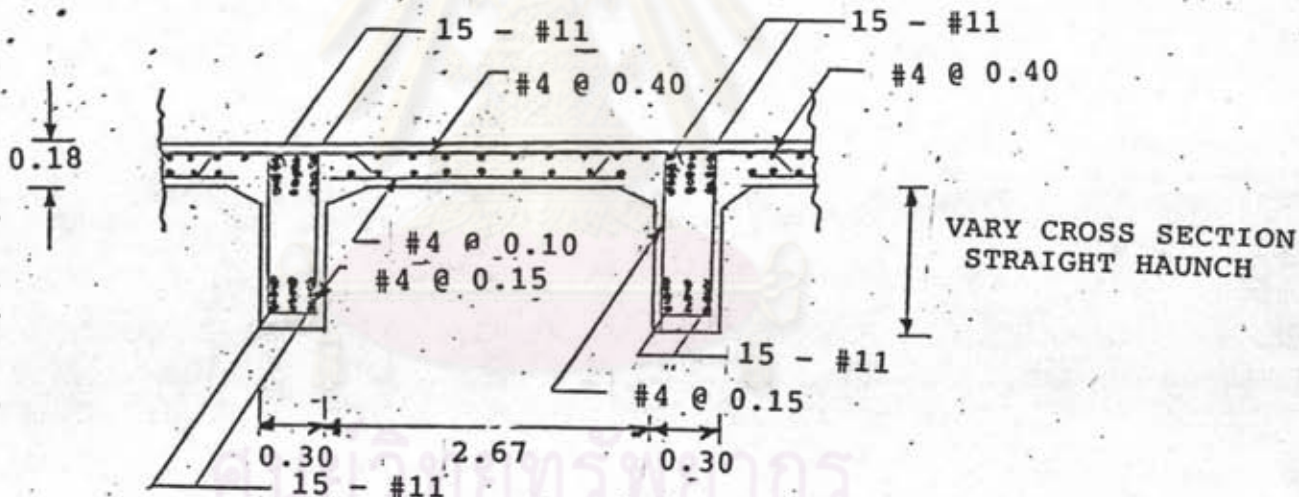
REACTION AT SUPPORT NO. 1	=	140	TON.
REACTION AT SUPPORT NO. 2	=	523	TON.
REACTION AT SUPPORT NO. 3	=	523	TON.
REACTION AT SUPPORT NO. 4	=	139	TON.

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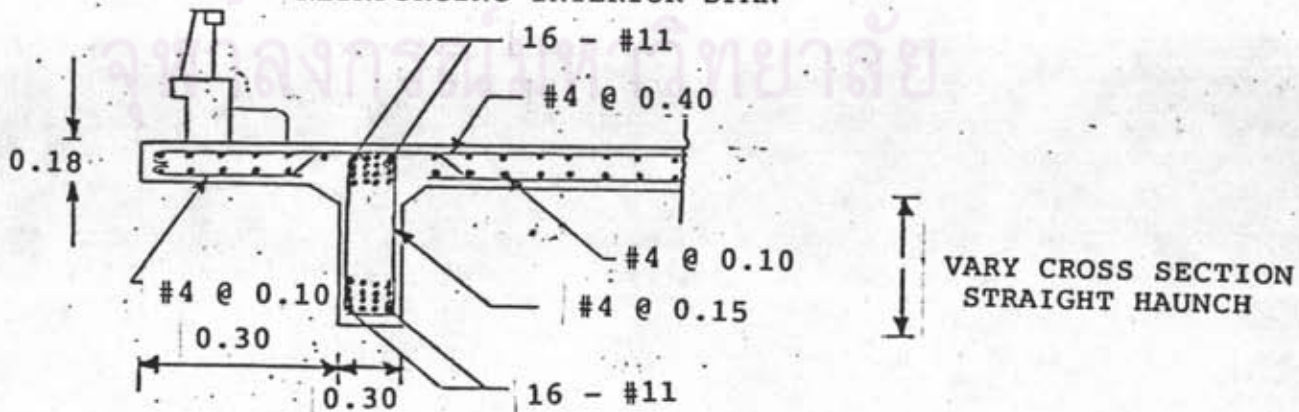
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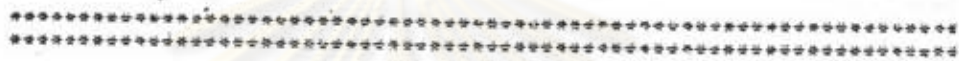
TYPICAL CROSS SECTION



REINFORCING INTERIOR SPAN

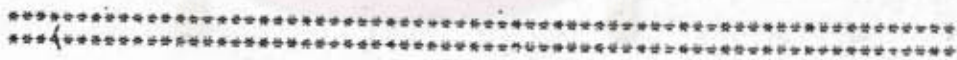


REINFORCING EXTERIOR SPAN



WWW	WW	WWW	WWW	WW	W	WWW	WW	WW	WW
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWW	W	WWW	WWW	WW	W	WWW	W	W	W
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWW	WW	WWW	WWW	WW	WW	WWW	WW	WW	WW

WW	W	W	WWW	W	WWW	W	W	WWW
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
WWW	WWW	W	WWW	WW	W	W	W	W



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\*\*\*\*\* TITLE READING LIST \*\*\*\*\*  
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An Existing Title is shown below

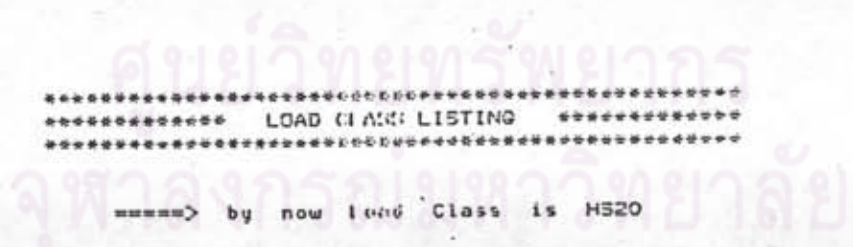
CONTINUOUS R.C. BEAM GIRDER BRIDGE DESIGN

\*\*\*\*\*  
\*\*\*\*\* MATERIAL PROPERTIES \*\*\*\*\*  
\*\*\*\*\*

- 1> Ultimate Strength of concrete 180.00 ksc.
- 2> Yield Strength of Steel 2500.00 ksc.

\*\*\*\*\*  
\*\*\*\*\* LOAD CLASS LISTING \*\*\*\*\*  
\*\*\*\*\*

====> by now load class is H520



\*\*\*\*\*  
 \*\*\*\*\* SETTLEMENT LISTING \*\*\*\*\*  
 \*\*\*\*\*

1> Settlement at Joint # 1 = 0.00 m.  
 2> Settlement at Joint # 2 = 0.01 m.  
 3> Settlement at Joint # 3 = 0.01 m.  
 4> Settlement at Joint # 4 = 0.00 m.

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\*\*\*\*\*  
 \*\*\*\*\* SLAB DESIGN \*\*\*\*\*  
 \*\*\*\*\*

LOAD = 16.0000  
 SPAN eff. = 95.1450  
 IMPACT MOMENT = 14297.4  
 DEAD WEIGHT = 15.6250  
 D. L. MOMENT = 1178.72  
 TOTAL MOMENT = 63134.1  
 d = 5.82725  
 REAL TOTAL DEPTH = 8.50000  
 DEAD WEIGHT = 121.875  
 D. L. MOMENT = 9194.01  
 TOTAL MOMENT = 71147.4  
 d = 6.18510  
 REAL TOTAL DEPTH = 8.50000  
 Actual d = 6.25000

\*\*\*\*\* TRANSVERSE STEEL \*\*\*\*\*

USE # 4 Spacing : 0.08 m.

\*\*\*\*\* LONGITUDINAL STEEL \*\*\*\*\*

USE # 3 Spacing : 0.05 m.

\*\*\*\*\* TEMPERATURE STEEL \*\*\*\*\*

USE # 4 Spacing : 0.46 m.

\*\*\*\*\*  
 \*\*\*\*\* EDGE DIMENSION LISTING \*\*\*\*\*  
 \*\*\*\*\*

1> Number of Traffic Lane	2
2> Lane width	3.00 m.
3> Outer dimension Apart Ex-Gir.	8.00 m.
4> Number of Girder per section	4
5> Overlapping Slab from Girder	0.30 m.
6> Area of Parapet Family	0.20 sq. m.
7> Number of span	3
8> Curb area	0.10 sq. m.
9> Cross section area	0.10 sq. m.

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\*\*\*\*\*  
 \*\*\*\*\* BEAM DIMENSION LISTING \*\*\*\*\*  
 \*\*\*\*\*

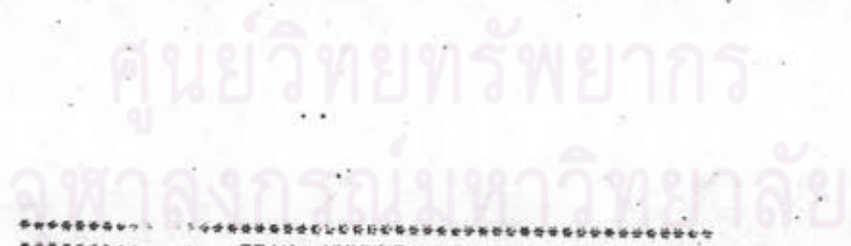
1> Beam Width	=	0.30 m.
2> Length of Interval	=	5.03 m.

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\*\*\*\*\*  
 \*\*\*\*\* SPAN NUMBER 1 \*\*\*\*\*  
 \*\*\*\*\*

====> now Code of Beam Shape = 2

Code : 1 = (straight haunch





2 = Parabolic haunch

3 = Constant Cross Section

1> Span Length = 30.18 m.  
 Number of Section Consider = 7 sect.  
 2> Left side Depth = 2.00 m.  
 3> Centre Depth = 1.80 m.  
 4> Right side Depth = 2.00 m.  
 5> Left side Length = 6.71 m.  
 6> Right side Length = 6.71 m.

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\*\*\*\*\*  
 \*\*\*\*\* SPAN NUMBER 2 \*\*\*\*\*  
 \*\*\*\*\*

====> by now Code of Beam Shape = 2

Code : 1 = Straight haunch  
 2 = Parabolic haunch  
 3 = Constant Cross Section

1> Span Length = 40.23 m.  
 Number of Section Consider = 7 sect.  
 2> Left side Depth = 2.00 m.  
 3> Centre Depth = 1.80 m.  
 4> Right side Depth = 2.00 m.  
 5> Left side Length = 6.71 m.  
 6> Right side Length = 6.71 m.

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 \*\*\*\*\* SPAN NUMBER 3 \*\*\*\*\*  
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====> by now Code of Beam Shape = 2

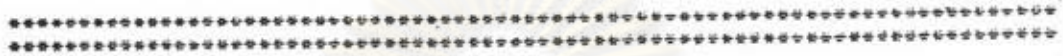
Code :        1 = Straight haunch  
               2 = Parabolic haunch  
               3 = Constant Cross Section

1> Span Length                =    30.18 m.  
     Number of Section Consider =        7 sect.  
 2> Left side Depth           =    2.00 m.  
 3> Centre      Depth        =    1.80 m.  
 4> Right side Depth         =    2.00 m.  
 5> Left side Length         =    6.71 m.  
 6> Right side Length        =    6.71 m.

\*\*\*\*\*  
 \*\*\*\*\* BAR LISTING \*\*\*\*\*  
 \*\*\*\*\*

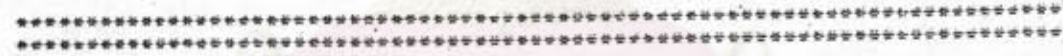
Bar #	1	2	3	4	5	6	7	8	9	10	11
Diam. (mm.)	4	6	9	12	16	19	25	28	30	33	36

1> BEAM Bar Size                #        11  
 2> BEAM Stirrup Bar Size      #        4  
 3> SLAB Bar Size                #        4



WWWW	WWW	WWWWW	WWW	WWW	W	WWWWW	WWW	WWW	WWW
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWWW	W	WWWW	WWW	WWW	W	W	W	WWW	WWW
W	W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W	W
WWWWW	WWW	WWWWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW

WWW	W	W	WWWWW	WWW	WWWWW	W	W	WWWWW
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W	W
W	W	W	W	W	WW	W	W	W
WWW	WWW	W	WWWWW	WWW	W	W	W	W



ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย

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 CONTINUOUS R. C. BEAM FOR BRIDGE DESIGN  
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NUMBER OF SPANS = 3  
 SPAN NO. 1 INTERVAL = 6  
 LENGTH = 99.0117  
 SPAN NO. 2 INTERVAL = 8  
 LENGTH = 131.9144  
 SPAN NO. 3 INTERVAL = 6  
 LENGTH = 99.0117

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 EXTERIOR-GIRDER  
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UNIFORM DEAD LOAD INTERVAL = 2107.71 KG/M  
 UNIFORM LIVE LOAD INTERVAL = 666.267 KG/M  
 CONCENTRATED LOAD EQUIVALENT = 8776.08 KG

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 GIRDER BENDING MOMENT-EQUIVALENT LOADING (TON-M)  
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	DEAD-LOAD	LIVE-LOAD	CONCEN. -LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL
1	88.3459	37.0868 -9.4174	0.0000	59.7597 -9.4174	13.3383 0.0000	-0.1207	161.3232 0.0000
2	119.1744	57.2582 -18.8348	0.0000	90.7493 -18.8348	20.2551 0.0000	-0.2413	227.9374 0.0000
3	92.8832	60.5141 -28.2521	0.0000	94.3541 -28.2521	21.0597 0.0000	-0.3620	207.9350 0.0000
4	9.4722	46.8546 -37.6695	-13.1076	72.9819 -50.7771	16.2895 -10.5565	-0.4826	98.2610 -52.3439
5	-131.0585	19.7800 -50.5874	-19.1428	33.3499 -69.7302	7.4437 -14.4968	-0.6033	0.0000 -215.8987
6	-329.1071	11.4841 -99.1995	-38.4674	16.5426 -137.6669	3.6923 -28.6207	-0.7239	0.0000 -496.1186
7	-128.8393	16.2285 -44.7543	-20.3809	30.9822 -65.1352	6.0280 -13.5415	-0.7239	0.0000 -208.2400
8	13.9252	44.9634 -31.2111	-12.8399	72.4397 -44.0510	14.0940 -9.1581	-0.7239	99.7349 -40.0079
9	99.5841	70.3303 -31.2111	-9.8568	106.5502 -41.0679	20.7306 -8.5380	-0.7239	226.1408 0.0000
10	128.1374	78.7859 -31.2110	0.0000	118.0191 -31.2110	22.9620 0.0000	-0.7239	268.3945 0.0000
11	99.5852	70.3302 -31.2110	-9.8568	106.5501 -41.0679	20.7305 -8.5380	-0.7239	226.1419 0.0000
12	13.9274	44.9633 -31.2110	-12.8399	72.4399 -44.0509	14.0940 -9.1581	-0.7239	99.7374 -40.0056
13	-128.8360	16.2286 -44.7545	-20.3808	30.9824 -65.1354	6.0280 -13.5415	-0.7239	0.0000 -208.2369
14	-329.1078	11.4842 -99.1997	-38.4674	16.5426 -137.6671	3.2186 -28.6208	-0.7239	0.0000 -496.1196
15	-131.0594	19.7800 -50.5877	-19.1428	33.3499 -69.7305	7.4437 -14.4969	-0.6033	0.0000 -215.8990
16	9.4716	46.8546 -37.6697	-13.1076	72.9820 -50.7773	16.2895 -10.5565	-0.4826	98.2604 -52.3449
17	92.8828	60.5142 -28.2523	0.0000	94.3542 -28.2523	21.0597 0.0000	-0.3620	207.9347 0.0000
18	119.1743	57.2583 -18.8349	0.0000	90.7493 -18.8349	20.2551 0.0000	-0.2413	227.9374 0.0000
19	88.3460	37.0869 -9.4174	0.0000	59.7598 -9.4174	13.3383 0.0000	-0.1207	161.3235 0.0000

UNIFORM DEAD LOAD INTERVAL = 2107.71 KG/M  
 UNIFORM LIVE LOAD INTERVAL = 666.267 KG/M  
 CONCENTRATED LOAD INTERVAL = 8776.08 KG

GIRDER SHEAR-EQUIVALENT LOADING (TON)

	DEAD-LOAD	LIVE-LOAD	CONCEN. -LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL		
1	17.5584	7.3709	-1.8717	13.6797	-1.8717	3.0979	0.0787	34.6147	0.0000
2	6.1270	4.7268	-2.5895	9.5341	-2.5895	2.1280	-0.5780	17.8678	0.0000
3	-5.2253	2.7740	-3.9986	6.0123	-3.9986	1.3419	-0.8925	2.2076	-10.0077
4	-16.5776	1.4586	-6.0451	3.3338	-12.0236	0.7441	-2.8837	0.0000	-31.2062
5	-27.9299	0.6969	-8.6452	1.4760	-15.9869	0.3294	-3.5682	0.0000	-47.4064
6	-39.3613	0.3804	-11.6906	0.6224	-20.1283	0.1389	-4.4926	0.0000	-63.9736
7	39.8122	13.1129	-1.3463	20.5947	-1.3463	4.0069	-0.2619	64.4139	0.0000
8	28.3809	10.0736	-1.6689	16.5639	-2.4632	3.2227	-0.4792	48.1674	0.0000
9	17.0286	7.4372	-2.3944	12.7859	-4.1802	2.4876	-0.8133	32.3021	0.0000
10	5.6762	5.2644	-3.5835	9.4025	-3.5835	1.8294	-0.6972	16.9081	0.0000
11	-5.6761	3.5835	-5.2644	6.5108	-10.2591	1.2668	-1.9960	2.1018	-17.9211
12	-17.0284	2.3944	-7.4372	4.1802	-13.6425	0.8133	-2.6543	0.0000	-33.3352
13	-28.3807	1.6689	-10.0737	2.5255	-17.4205	0.4914	-3.3844	0.0000	-49.1906
14	-39.8130	1.3463	-13.1129	2.2029	-21.4514	0.4286	-4.1736	0.0000	-65.4780
15	39.3613	11.6906	-0.3804	19.1876	-0.3804	4.2826	-0.0849	62.7528	0.0000
16	27.9299	8.6452	-0.6969	15.0461	-1.4760	3.3583	-0.3294	46.2587	0.0000
17	16.5776	6.0451	-1.4586	11.0829	-1.4586	2.4737	-0.3256	30.0555	0.0000
18	5.2253	3.9986	-2.7740	7.4674	-2.7740	1.6667	-0.6192	14.2808	0.0000
19	-6.1270	2.5895	-4.7268	4.3567	-4.7268	0.9724	-1.0550	0.0000	-11.9879
20	-18.7407	1.8717	-7.3709	2.8124	-7.3709	0.6277	-1.6452	0.0000	-27.8354

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INTERIOR-GIRDER  
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UNIFORM DEAD LOAD INTERVAL = 2446.08 KG/M  
UNIFORM LIVE LOAD INTERVAL = 661.167 KG/M  
CONCENTRATED LOAD EQUIVALA = 5779.08 KG

GIRDER BENDING MOMENT-EQUIVALENT LOADING (TON-M)

	DEAD-LOAD	LIVE-LOAD	CONCEN. -LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL			
1	86.1854	37.0868	-9.4174	59.7597	-9.4174	13.3383	0.0000	-0.1207	157.1627	0.0000
2	116.2599	57.2582	-18.8348	90.7493	-18.8348	20.2551	0.0000	-0.2413	227.0230	0.0000
3	90.6117	60.5141	-28.2521	94.3541	-28.2521	21.0597	0.0000	-0.3620	205.6635	0.0000
4	9.2406	46.0546	-37.6695	72.9819	-50.7771	16.2895	-10.5565	-0.4826	98.0288	-52.5763
5	-127.8534	19.7800	-50.5874	33.3499	-69.7302	7.4437	-14.4968	-0.6033	0.0000	-212.6837
6	-321.0588	11.4841	-99.1995	16.5426	-137.6669	3.6923	-28.6207	-0.7239	0.0000	-488.0703
7	-125.6885	16.2285	-44.7543	30.9822	-65.1352	6.0280	-13.5415	-0.7239	0.0000	-205.0892
8	13.5846	44.9634	-31.2111	72.4397	-44.0510	14.0940	-9.1581	-0.7239	99.3944	-40.3485
9	97.1487	70.3303	-31.2111	106.5502	-41.0679	20.7306	-8.5380	-0.7239	223.7055	0.0000
10	125.0038	78.7859	-31.2110	118.0191	-31.2110	22.9620	0.0000	-0.7239	265.2609	0.0000
11	97.1499	70.3302	-31.2110	106.5501	-41.0679	20.7305	-8.5380	-0.7239	223.7066	0.0000
12	13.5868	44.9633	-31.2110	72.4399	-44.0509	14.0940	-9.1581	-0.7239	99.3968	-40.3482
13	-125.6853	16.2285	-44.7545	30.9824	-65.1354	6.0280	-13.5415	-0.7239	0.0000	-205.0862
14	-321.0594	11.4842	-99.1997	16.5426	-137.6671	3.2186	-28.6208	-0.7239	0.0000	-488.0713
15	-127.8543	19.7800	-50.5877	33.3499	-69.7305	7.4437	-14.4969	-0.6033	0.0000	-212.6850
16	9.2400	46.8546	-37.6697	72.9820	-50.7773	16.2895	-10.5565	-0.4826	98.0288	-52.5763
17	90.6113	60.5142	-28.2523	94.3542	-28.2523	21.0597	0.0000	-0.3620	205.6632	0.0000
18	116.2599	57.2583	-18.8349	90.7493	-18.8349	20.2551	0.0000	-0.2413	227.0230	0.0000
19	86.1855	37.0869	-9.4174	59.7598	-9.4174	13.3383	0.0000	-0.1207	157.1630	0.0000

UNIFORM DEAD LOAD INTERVAL= 2400.001 KG/M  
 UNIFORM LIVE LOAD INTERVAL= 600.007 KG/M  
 CONCENTRATED LOAD INTERVAL= 8770.000 KG

GIRDER SHEAR-EQUIVALENT LOADING (TON)

	DEAD-LOAD	LIVE-LOAD	CINCENT. -LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	17.1290	7.3709	-1.8717	6.5009	0.0000	13.8797	-1.8717	3.0979	-0.4178	0.0787	34.1853	0.0000
2	5.9772	4.7268	-2.5895	4.1003	0.0000	9.5341	-2.5895	2.1280	-0.5780	0.0787	17.7179	0.0000
3	-5.0975	2.7740	-3.9986	5.7002	0.0000	6.0123	-3.9986	1.3419	-0.8925	0.0787	2.3394	-9.9100
4	-16.1722	1.4586	-6.0451	1.0051	-5.9785	3.3336	-12.0236	0.7441	-2.6837	0.0787	0.0000	-30.8008
5	-27.2469	0.6969	-8.6452	0.7791	-7.3416	1.4760	-15.9869	0.3294	-3.5682	0.0787	0.0000	-46.7234
6	-38.3987	0.3804	-11.6906	0.7470	-8.4377	0.6224	-20.1283	0.1387	-4.4926	0.0787	0.0000	-62.7410
7	38.8386	13.1129	-1.3463	7.4008	0.0000	20.5947	-1.3463	4.0069	-0.2619	0.0000	63.4403	0.0000
8	27.6868	10.0736	-1.6689	6.4902	-0.7942	16.5639	-2.4632	3.2227	-0.4792	0.0000	47.4734	0.0000
9	16.6121	7.4372	-2.3944	5.1007	-1.7858	12.7859	-4.1802	2.4876	-0.8133	0.0000	31.8857	0.0000
10	5.5374	5.2644	-3.5835	4.1000	0.0000	9.4025	-3.5835	1.8294	-0.6972	0.0000	16.7693	0.0000
11	-5.5373	3.5835	-5.2644	7.9773	-4.9946	6.5108	-10.2591	1.2669	-1.9960	0.0000	2.2403	-17.7923
12	-16.6120	2.3944	-7.4372	1.7008	-6.2053	4.1802	-13.6425	0.8133	-2.6543	0.0000	0.0000	-32.9088
13	-27.6867	1.6689	-10.0737	0.0000	-7.3468	2.5255	-17.4205	0.4914	-3.3894	0.0000	0.0000	-48.4965
14	-38.8394	1.3463	-13.1129	0.0000	-8.3384	2.2029	-21.4514	0.4286	-4.1736	0.0000	0.0000	-64.4644
15	38.3987	11.6906	-0.3804	7.4009	0.0000	19.1876	-0.3804	4.2826	-0.0849	-0.0787	61.7902	0.0000
16	27.2469	8.6452	-0.6969	6.4009	-0.7791	15.0461	-1.4760	3.3583	-0.3294	-0.0787	45.5727	0.0000
17	16.1722	6.0451	-1.4586	5.0378	0.0000	11.0829	-1.4586	2.4737	-0.3296	-0.0787	29.6501	0.0000
18	5.0975	3.9986	-2.7740	3.4008	0.0000	7.4674	-2.7740	1.6067	-0.6192	-0.0787	14.1530	0.0000
19	-5.9772	2.5895	-4.7268	1.7072	0.0000	4.3567	-4.7268	0.9724	-1.0550	-0.0787	0.0000	-11.8377
20	-18.2824	1.8717	-7.3709	0.9407	0.0000	2.8124	-7.3709	0.6277	-1.6452	-0.0787	0.0000	-27.3771

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 TRUCK EXTERIOR GIRDER  
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GIRDER BENDING MOMENT-TRUCK LOADING (TON-M)

	DEAD-LOAD	HI-AXIAL LOAD	HIW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	88.3459	3.5825	-0.5184	1.0008	-0.2572	62.5954	-9.3240	13.9712	0.0000	-0.1207	164.7918	0.0000
2	119.1744	5.2891	-1.0369	7.6301	-0.5144	75.1463	-18.6481	21.2365	0.0000	-0.2413	235.3159	0.0000
3	92.8832	5.3425	-1.5553	7.6612	-0.7717	76.1244	-27.9721	21.4548	0.0000	-0.3620	210.1004	0.0000
4	9.4722	4.1246	-3.5433	7.0493	-1.7556	74.1900	-63.7183	16.5591	-13.2463	-0.4826	99.7388	-67.9720
5	-131.0585	2.1364	-5.5612	1.0073	-2.7560	38.4111	-100.0039	8.5733	-20.7907	-0.6033	0.0000	-252.4563
6	-329.1071	0.8008	-7.5792	0.7791	-3.7564	14.4096	-136.2925	3.2162	-28.3350	-0.7239	0.0000	-494.4564
7	-128.8393	2.3237	-2.5050	1.7075	-1.2485	41.7764	-45.0739	8.1281	-9.3708	-0.7239	0.0000	-184.0079
8	13.9252	4.3381	-2.0327	7.1008	-1.0131	78.0326	-36.5762	15.1821	-7.6041	-0.7239	106.4160	-30.9791
9	99.5841	5.7161	-1.5605	7.0005	-0.7777	102.7000	-28.0786	19.9815	-5.8375	-0.7239	221.5416	0.0000
10	128.1374	4.1070	-1.4327	7.0162	-0.6798	73.7764	-25.6427	14.3540	0.0000	-0.7239	215.5439	0.0000
11	99.5852	2.4978	-1.5605	1.7079	-0.7777	44.8528	-28.0785	8.7266	-5.8375	-0.7239	152.4407	0.0000
12	13.9274	1.8831	-2.0327	0.9303	-1.0131	33.8502	-36.5761	6.5859	-7.6041	-0.7239	53.6396	-30.9788
13	-128.8360	2.3175	-2.5050	1.7071	-1.2485	41.6049	-45.0738	8.0947	-9.3708	-0.7239	0.0000	-184.0048
14	-329.1078	0.8008	-4.1491	0.7771	-2.0509	14.4097	-74.5899	2.8036	-15.5071	-0.7239	0.0000	-419.9288

15	-131.0594	2.1364	-3.4576	1.0073	-1.7091	38.4111	-62.1554	8.5733	-12.7226	-0.6033	0.0000	-206.7437
16	9.4716	4.1246	-2.7661	2.0473	-1.3673	74.1901	-49.7267	16.5591	-10.3381	-0.4026	99.7381	-31.0759
17	92.8828	5.3425	-2.0746	2.6612	-1.0255	96.1244	-37.2931	21.4548	0.0000	-0.3620	210.1000	0.0000
18	119.1743	5.2891	-1.3830	2.6301	-0.6836	95.1464	-24.8634	21.2365	0.0000	-0.2413	235.3159	0.0000
19	88.3460	3.5825	-0.6915	1.0108	-0.3418	62.5954	-12.4317	13.9712	0.0000	-0.1207	164.7920	0.0000

GIRDER SHEAR—TRUCK LOADING (TON)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	17.5584	0.7729	-0.1030	0.3667	-0.0495	13.8971	-1.8466	3.1018	-0.4122	0.0787	34.6359	0.0000
2	6.1270	0.5256	-0.1974	0.2078	-0.0978	9.4406	-3.5501	2.1071	-0.7924	0.0787	17.7534	0.0000
3	-5.2253	0.3538	-0.3788	0.1717	-0.1843	6.3471	-6.7973	1.4167	-1.5172	0.0787	2.6171	-13.4611
4	-16.5776	0.2046	-0.5509	0.0768	-0.2701	3.6606	-9.8943	0.8170	-2.2084	0.0787	0.0000	-28.6017
5	-27.9299	0.1561	-0.7005	0.0766	-0.3446	2.8032	-12.5858	0.6257	-2.8091	0.0787	0.0000	-43.2462
6	-39.3613	0.1561	-0.8208	0.0766	-0.4044	2.8032	-14.7510	0.6257	-3.2924	0.0787	0.0000	-57.3261
7	39.8122	0.8193	-0.0939	0.4036	-0.0468	14.7227	-1.6893	2.8645	-0.3287	0.0000	57.3994	0.0000
8	28.3809	0.7460	-0.0939	0.3700	-0.0468	13.4152	-1.6893	2.6101	-0.3287	0.0000	44.4061	0.0000
9	17.0286	0.5817	-0.1949	0.2470	-0.0917	10.4544	-3.4850	2.0340	-0.6780	0.0000	29.5170	0.0000
10	5.6762	0.0939	-0.3646	0.0468	-0.1801	1.6893	-6.5535	0.3287	-1.2751	0.0000	7.6942	-2.1523
11	-5.6761	0.0939	-0.3646	0.0468	-0.1801	1.6893	-6.5535	0.3287	-1.2751	0.0000	0.0000	-13.5046
12	-17.0284	0.0939	-0.3646	0.0468	-0.1801	1.6893	-6.5535	0.3287	-1.2751	0.0000	0.0000	-24.8369
13	-28.3807	0.0939	-0.3646	0.0468	-0.1801	1.6893	-6.5535	0.3287	-1.2751	0.0000	0.0000	-36.2092
14	-39.8130	0.0939	-0.8166	0.0468	-0.4036	1.6893	-14.6791	0.3287	-2.8560	0.0000	0.0000	-57.3481
15	39.3613	0.8208	-0.1561	0.4044	-0.0768	14.7510	-2.8040	3.2924	-0.6257	-0.0787	57.3260	0.0000
16	27.9299	0.7005	-0.1561	0.3446	-0.0768	12.5858	-2.8040	2.8091	-0.6257	-0.0787	43.2462	0.0000
17	16.5776	0.5509	-0.2046	0.2701	-0.0968	9.8943	-3.6606	2.2084	-0.8170	-0.0787	28.6017	0.0000
18	5.2253	0.3788	-0.3538	0.1843	-0.1717	6.7973	-6.3471	1.5172	-1.4167	-0.0787	13.4611	-2.6171
19	-6.1270	0.1922	-0.5256	0.0911	-0.2578	3.4394	-9.4406	0.7677	-2.1071	-0.0787	0.0000	-17.7534
20	-18.7407	0.0772	-0.7120	0.0468	-0.3511	1.3820	-12.7963	0.3085	-2.8562	-0.0787	0.0000	-34.4720

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TRUCK INTERIOR GIRDER  
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GIRDER BENDING MOMENT—TRUCK LOADING (TON-M)

	DEAD-LOAD	HI-AXIAL LOAD	LOW-AXIAL LOAD	TOTAL LL.	IMPACT LOAD	SETTLE	TOTAL					
1	86.1854	3.5825	-0.5184	1.0108	-0.2572	62.5954	-9.3240	13.9712	0.0000	-0.1207	162.6313	0.0000
2	116.2599	5.2891	-1.0369	2.6301	-0.5144	95.1463	-18.6481	21.2365	0.0000	-0.2413	232.4015	0.0000
3	90.6117	5.3425	-1.5553	2.6612	-0.7717	96.1244	-27.9721	21.4548	0.0000	-0.3620	207.8289	0.0000
4	9.2406	4.1246	-3.5433	2.0473	-1.7556	74.1900	-63.7153	16.5591	-13.2463	-0.4026	99.5071	-68.2036
5	-127.8534	2.1364	-5.5612	1.0073	-2.7560	38.4111	-100.0039	8.5733	-20.7907	-0.6033	0.0000	-249.2312
6	-321.0588	0.8008	-7.5792	0.3721	-3.7564	14.4096	-136.2925	3.2162	-28.3350	-0.7239	0.0000	-466.4102
7	-125.6885	2.3237	-2.5050	1.1495	-1.2485	41.7764	-45.0739	8.1281	-9.3708	-0.7239	0.0000	-180.8971
8	13.5846	4.3381	-2.0327	2.1568	-1.0131	78.0326	-36.5762	15.1821	-7.6041	-0.7239	106.0734	-31.3147
9	97.1487	5.7161	-1.5605	2.0105	-0.7777	102.7000	-28.0786	19.9815	-5.8375	-0.7239	217.1063	0.0000
10	125.0038	4.1070	-1.4327	2.0162	-0.6798	73.7764	-25.6427	14.3540	0.0000	-0.7239	212.4103	0.0000
11	97.1499	2.4978	-1.5605	1.2219	-0.7777	44.8528	-28.0785	8.7266	-5.8375	-0.7239	150.0053	0.0000
12	13.5868	1.8831	-2.0327	0.2103	-1.0131	33.8502	-36.5761	6.5859	-7.6041	-0.7239	53.2990	-31.3174
13	-125.6853	2.3175	-2.5050	1.1491	-1.2485	41.6049	-45.0738	8.0947	-9.3708	-0.7239	0.0000	-180.8538

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WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW
W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W
WWW	W	WWW	WWW	WWW	W	W	W
W	W	W	W	W	W	W	W
W	W	W	W	W	W	W	W
WWW	WWW	WWW	WWW	WWW	WWW	WWW	WWW

WWW	W	W	WWW	WWW	W	WWW
W	W	W	W	W	W	W
W	W	W	W	W	W	W
W	W	W	W	W	W	W
W	W	W	W	W	W	W
WWW	WWW	W	WWW	WWW	W	WWW

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ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย



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 \*\*\*\*\* BEAM DESIGN \*\*\*\*\*  
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\*\*\*\*\*  
 EXTERIOR DESIGN  
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Span # 1

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	2	2	0.40
2	5.03	1.81	2	5	37.33
3	10.06	1.80	2	7	0.87
4	15.09	1.80	2	7	0.40
5	20.12	1.80	2	3	0.18
6	25.15	1.81	7	7	0.12
7	30.18	2.00	16	16	

Span # 2

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrups # 4 (m.)
1	0.00	2.00	16	16	0.14
2	5.03	1.81	7	5	0.18
3	10.06	1.80	2	3	0.37
4	15.09	1.80	2	7	0.87
5	20.12	1.80	2	9	18.33
6	25.15	1.80	2	7	0.35
7	30.18	1.80	2	3	0.17
8	35.21	1.81	7	5	

9      40.24      2.00      16      16      0.12

Span #      3

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	16	16	0.14
2	5.03	1.81	7	5	0.19
3	10.06	1.80	2	3	0.43
4	15.09	1.80	2	7	0.89
5	20.12	1.80	2	7	34.25
6	25.15	1.81	2	5	0.33
7	30.18	2.00	2	2	

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 INTERIOR DESIGN  
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Span #      1

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	2	2	0.45
2	5.03	1.81	2	5	0.90
3	10.06	1.80	2	7	0.87
4	15.09	1.80	2	6	0.45
5	20.12	1.80	2	3	0.20
6	25.15	1.81	8	7	0.13
7	30.18	2.00	15	13	

Span # 2

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	15	15	
2	5.03	1.81	7	4	0.15
3	10.06	1.80	2	3	0.20
4	15.09	1.80	2	7	0.41
5	20.12	1.80	2	8	0.89
6	25.15	1.80	2	7	0.89
7	30.18	1.80	2	3	0.39
8	35.21	1.81	7	4	0.19
9	40.24	2.00	15	15	0.12

Span # 3

Section No.	Distance (m.)	Depth (m.)	Top Bar # 11	Bottom Bar # 11	Stirrub # 4 (m.)
1	0.00	2.00	15	15	
2	5.03	1.81	7	3	0.15
3	10.06	1.80	2	3	0.21
4	15.09	1.80	2	6	0.49
5	20.12	1.80	2	7	0.89
6	25.15	1.81	2	9	0.89
7	30.18	2.00	2	2	0.36

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QUANTITY OF CONCRETE (cu. m)

Slab (cu. m)	Girder (cu. m)	Total (cu. m)
153.81	221.38	375.19

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QUANTITY OF FORMWORK (sq. m)

Slab (sq. m)	Girder (sq. m)	Total (sq. m)
780.12	1596.57	2376.70

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QUANTITY OF STEEL (ton)

Slab (ton)	Girder (ton)	Total (ton)
1.93	43.85	45.78

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## DEFLECTION EACH SPAN (MIDDLE SPAN) (CM.)

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DEF. FOR SPAN 1 =	2.887	CM.
DEF. FOR SPAN 2 =	7.172	CM.
DEF. FOR SPAN 3 =	2.887	CM.

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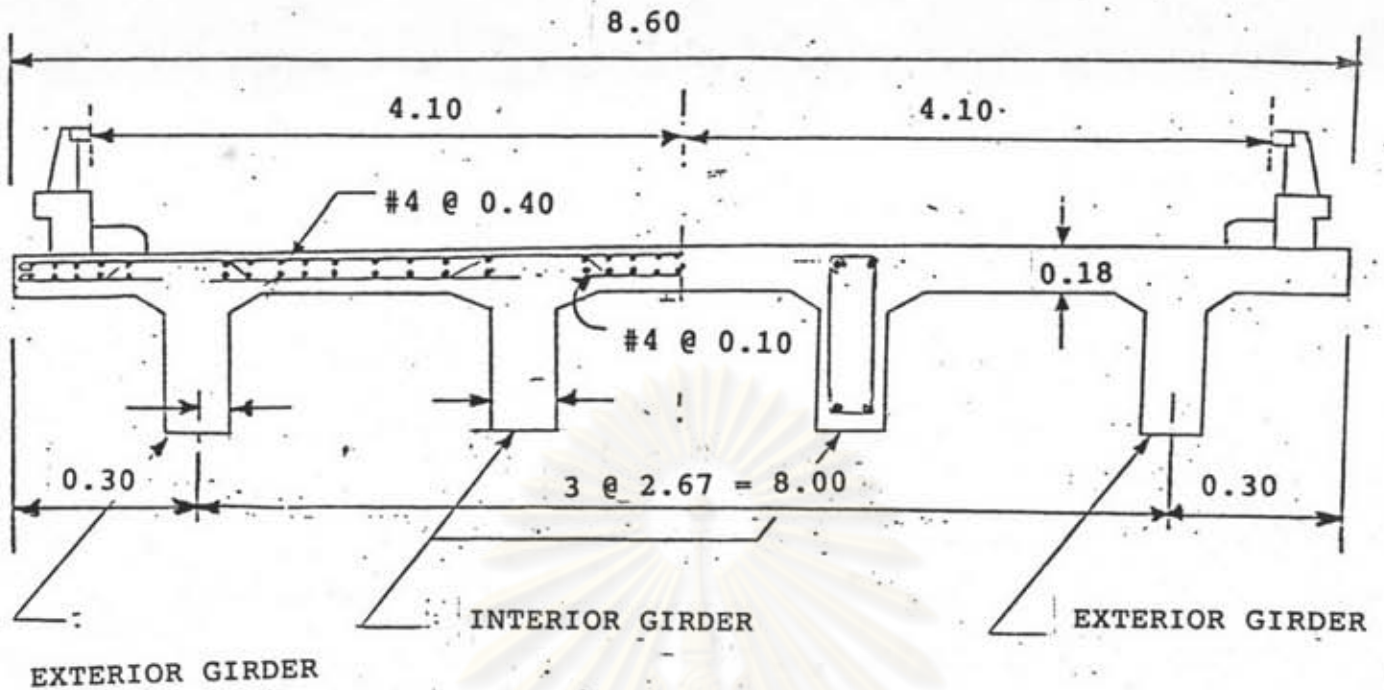
## REACTION AT SUPPORT (TON.)

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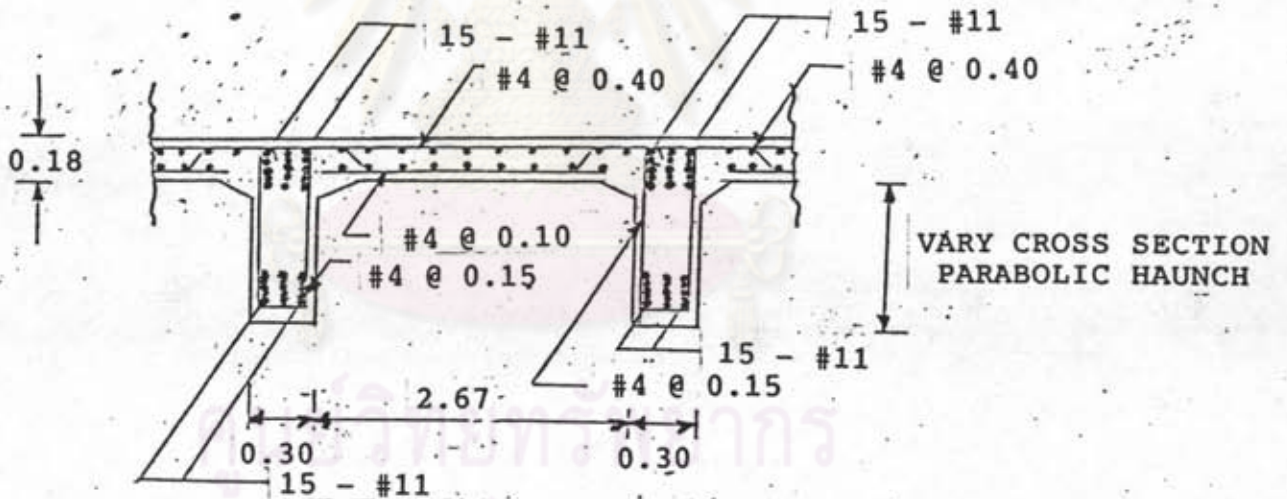
REACTION AT SUPPORT NO. 1=	140	TON.
REACTION AT SUPPORT NO. 2=	520	TON.
REACTION AT SUPPORT NO. 3=	520	TON.
REACTION AT SUPPORT NO. 4=	139	TON.

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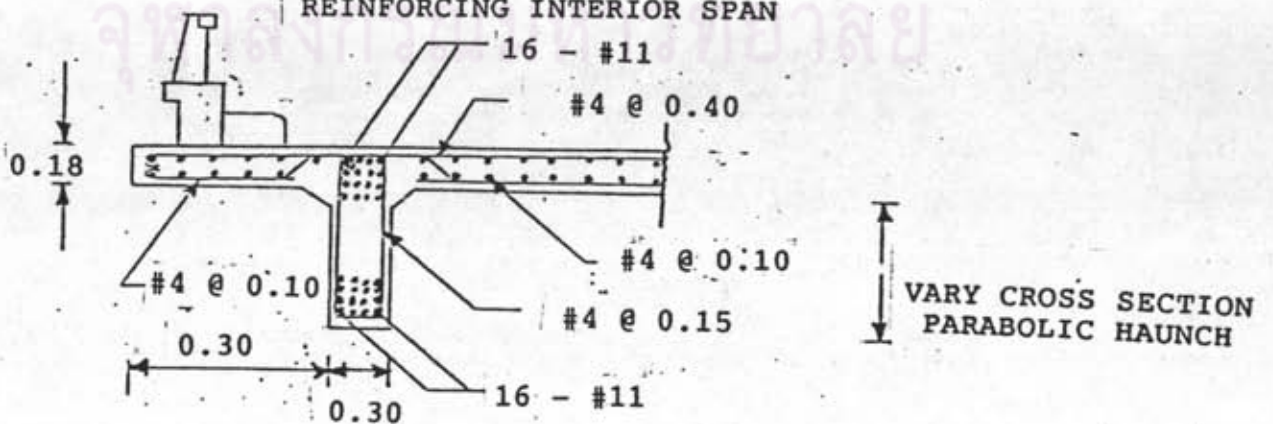
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จุฬาลงกรณ์มหาวิทยาลัย



TYPICAL CROSS SECTION



REINFORCING INTERIOR SPAN



REINFORCING EXTERIOR SPAN



ประวัติผู้เขียน.

นายชาญวิทย์ อัจฉิมิตี เกิดวันที่ 15 ตุลาคม พ.ศ. 2504 ที่อำเภอลาดบัวหลวง  
จังหวัดพระนครศรีอยุธยา สำเร็จการศึกษา ระดับปริญญาตรีวิศวกรรมศาสตรบัณฑิต สาขาวิศวกรรม  
โยธา คณะวิศวกรรมศาสตร์ จากมหาวิทยาลัยขอนแก่น เมื่อปีการศึกษา 2527



ศูนย์วิทยทรัพยากร  
จุฬาลงกรณ์มหาวิทยาลัย