## CHAPTER V

## ANALYSIS OF RESULTS

The relationship of the tests can be summerized as follows :-

Load - settlement of all piles are shown from Fig
23 to Fig 25

2. Load - top and toe settlement of instrumentation piles are shown from Fig 26 to Fig 28

3. The relationship of load to plastic and elastic settlement from Fig 29 to Fig 34

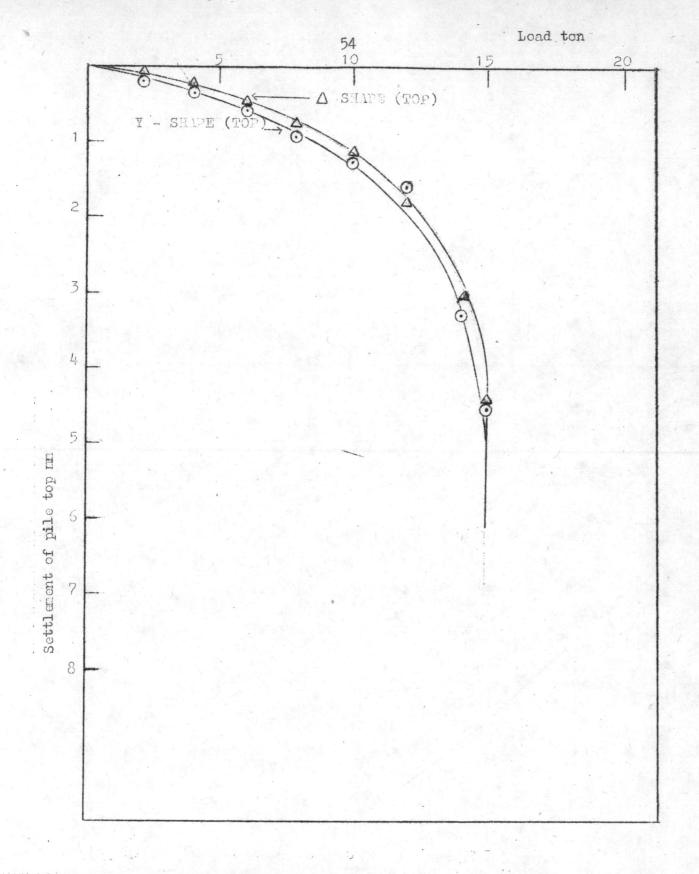
4. The relationship of load-ratio to total, plastic, elastic settlement of all piles from Fig 35 to Fig 40

5. The quick test of all piles from Fig 41 to 46

6. The soil properties at the test site of long pile,

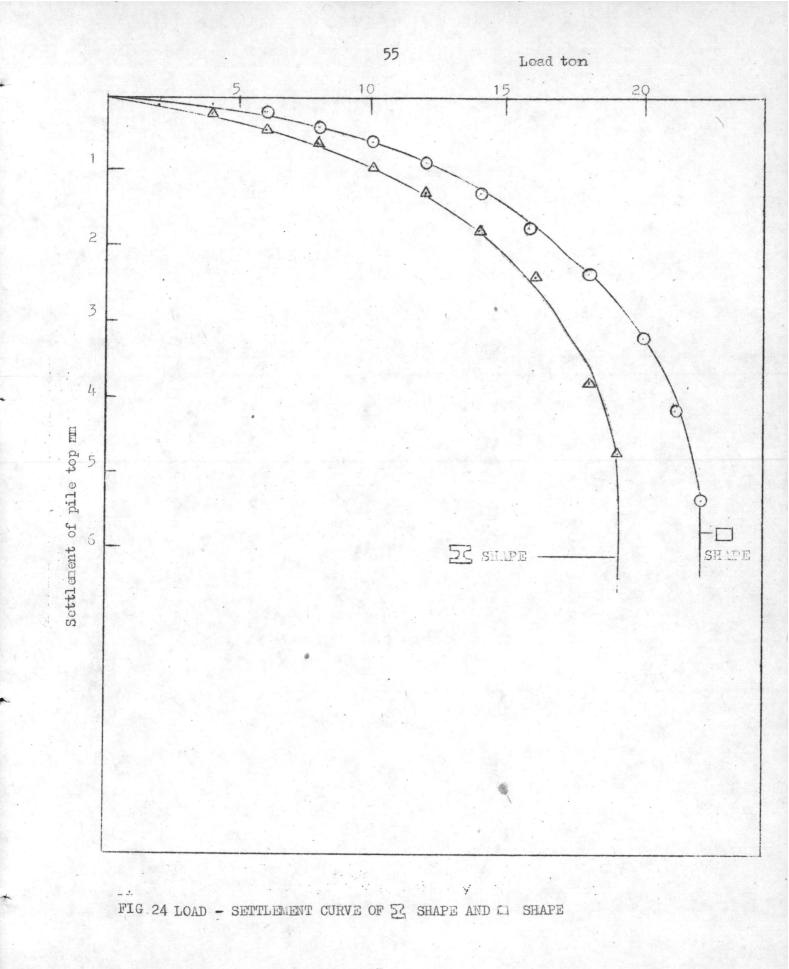
load-time relationship, load and total, plastic, elastic settlement relationship are shown from **Fig** 47 to Fig 53

Summary of all relationships are collected in table
to table 6



LOAD - SETTLEMENT CURVE OF Y SHAPE AND  $\triangle$  SHAPE

FIG 23



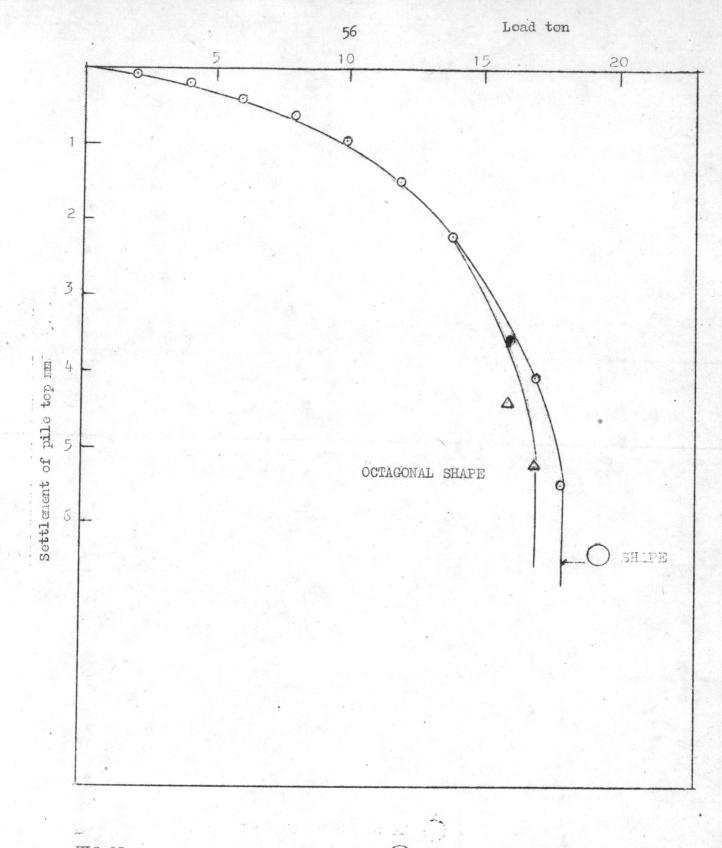
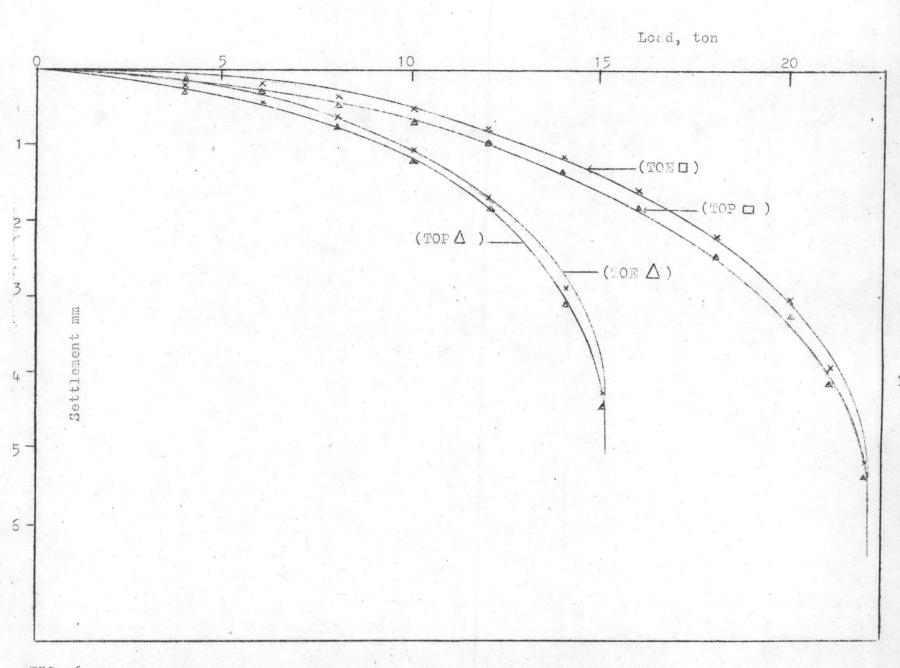


FIG 25 LOAD - SETTLEMENT CURVE OF O SHAPE AND OCTAGONAL SHAPE

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FIG 26 RELATIONSHIP OF LOAD AND TOP, TOE SETTLEMENT OF  $\square$  AND  $\triangle$  SHAPE

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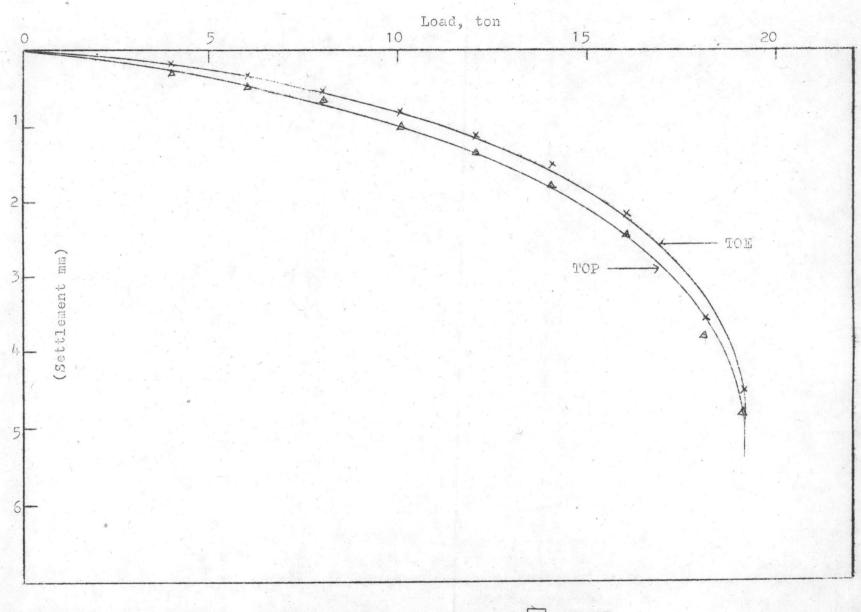
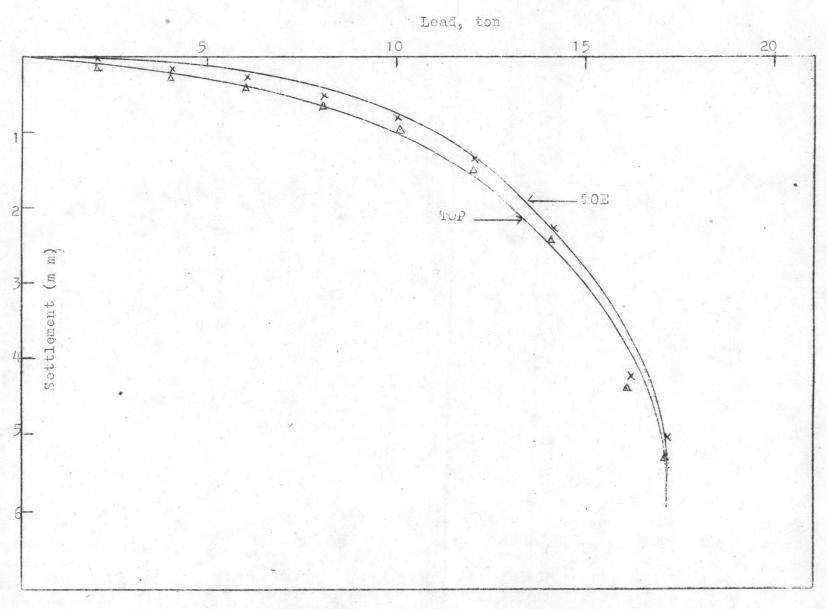


FIG 27 RELATIONSHIP OF LOAD AND TOP, TOE SETTLEM MY OF 2 SHAPE

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FIG 28 RELATIONSHIP OF LOID AND TOP, TOE SEATLEMELT OF OCTAGONAL SHAPE

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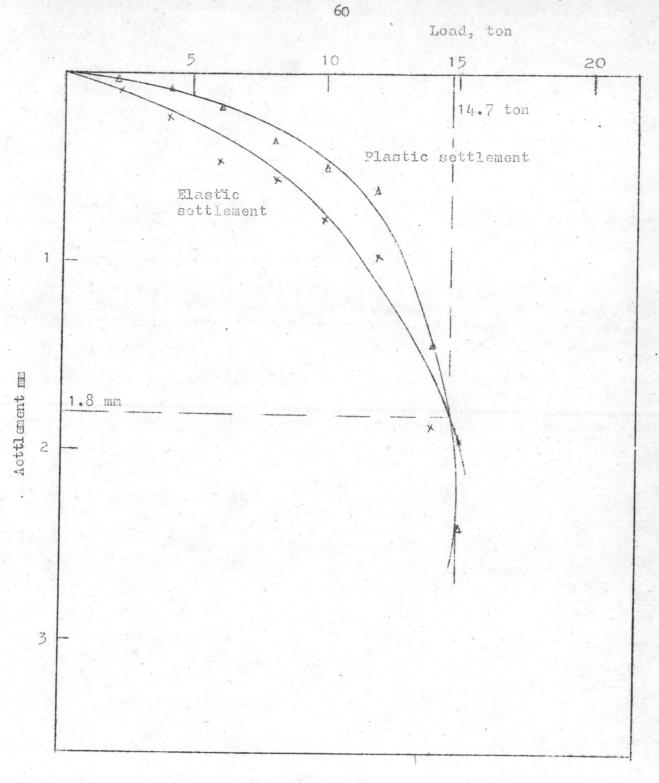
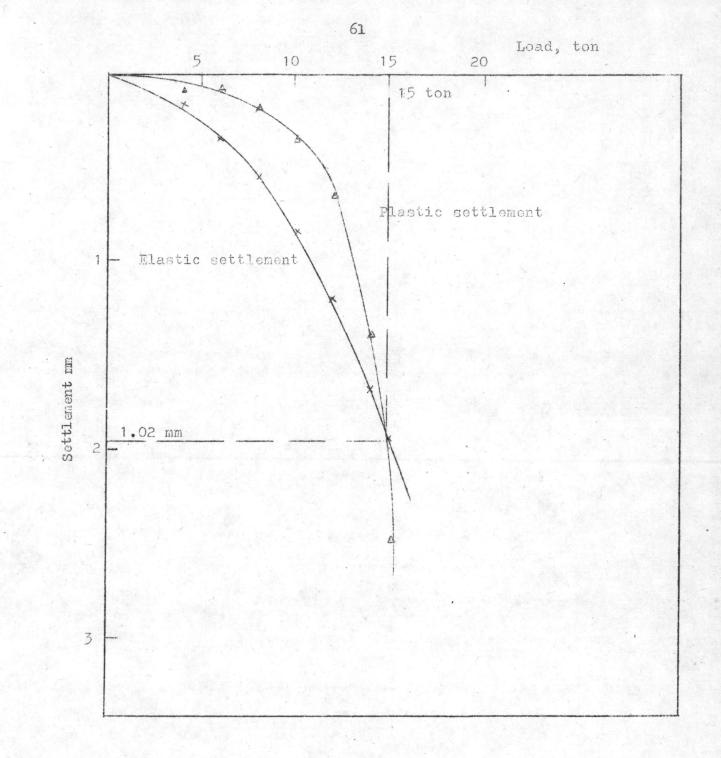
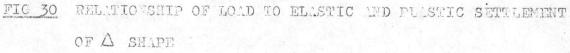
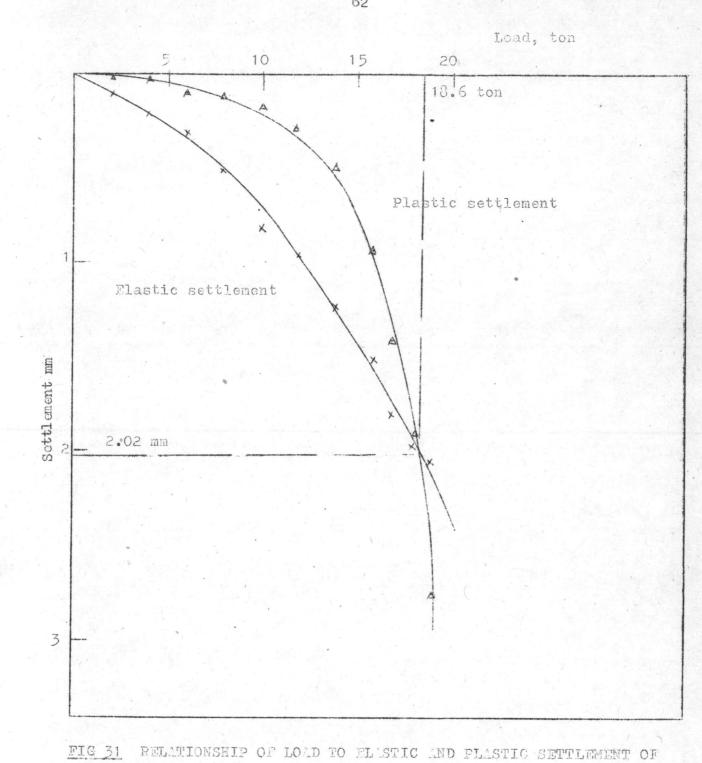


FIG 29 RELATIONSHIP OF LOAD TO ELISTIC AND PLASTIC SETTLEMENT OF  $\gamma$  shape



i.





53 SHAPE

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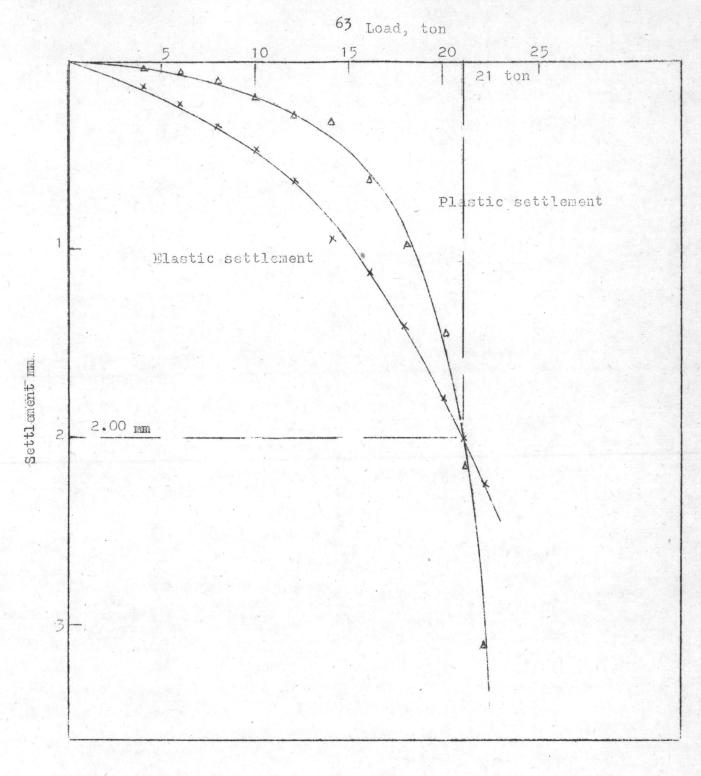
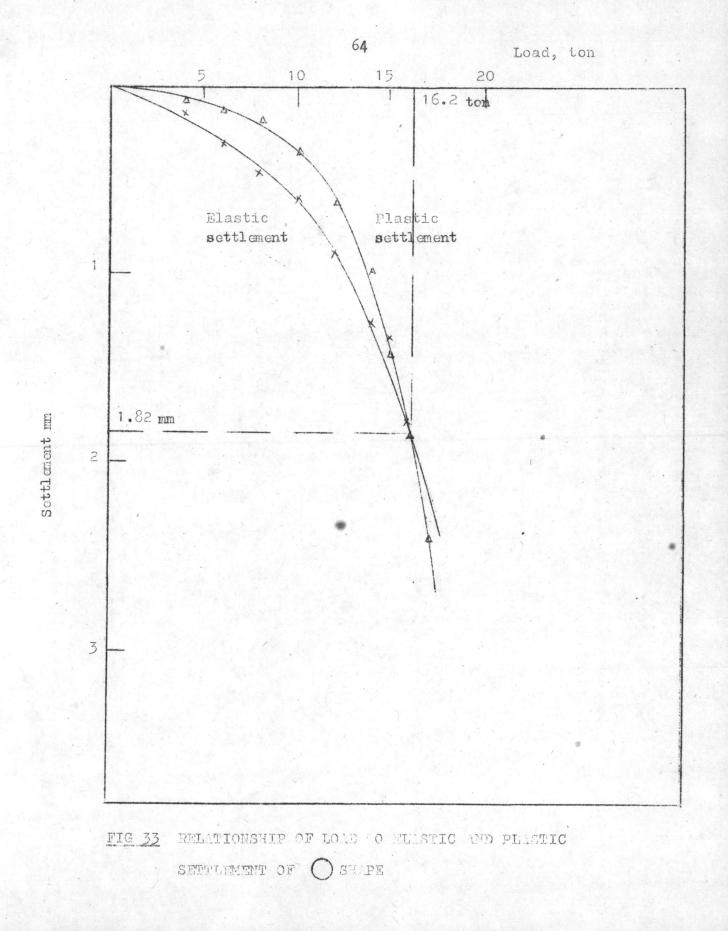
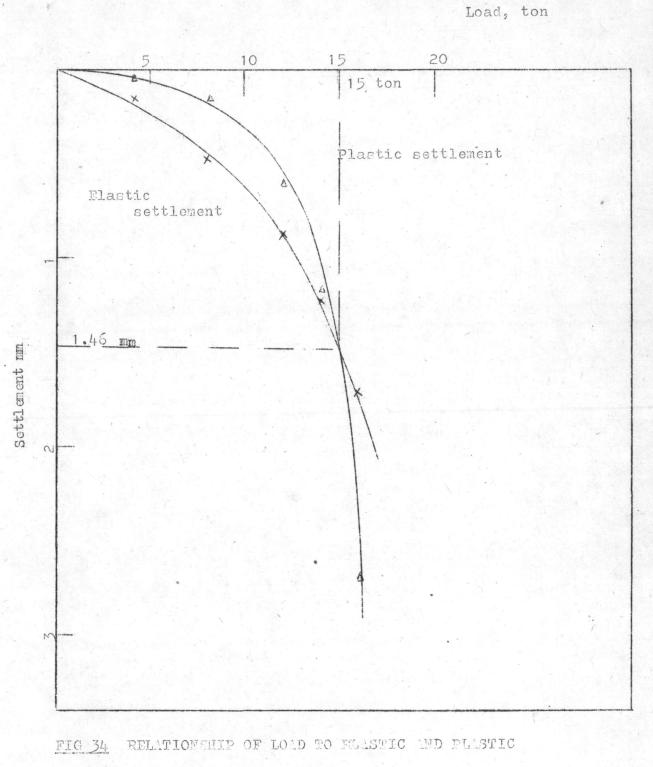


FIG 32 RELATIONSHIP OF LOAD TO PLASFIC AND PLASEIC SETTLEMENT OF

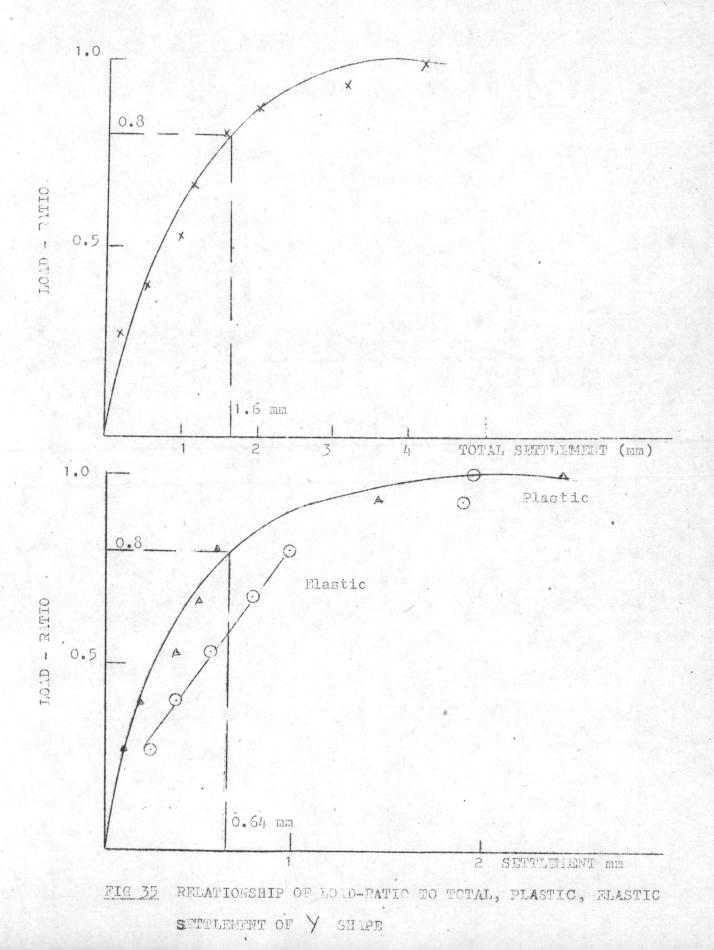
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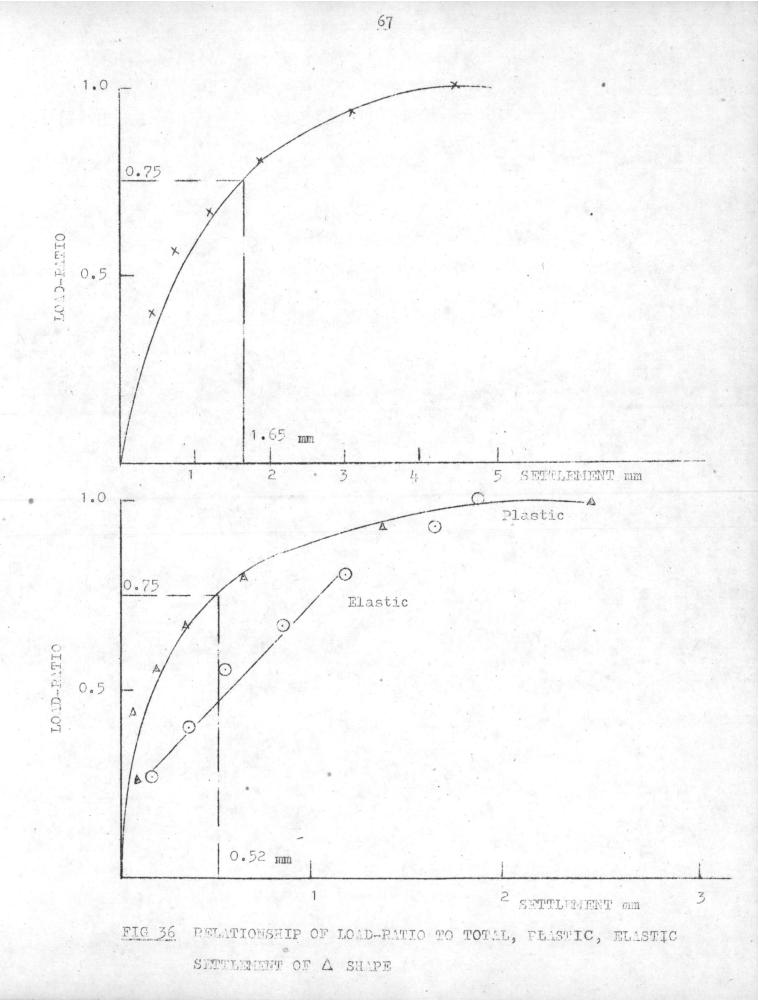


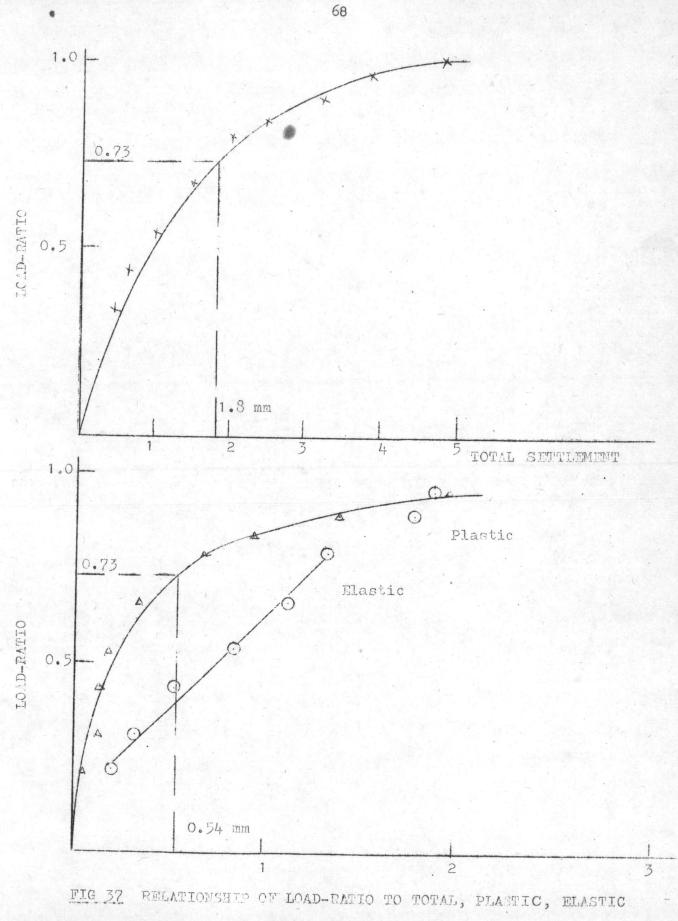


SETTLEMENT OF OCTAGOFAL PILE

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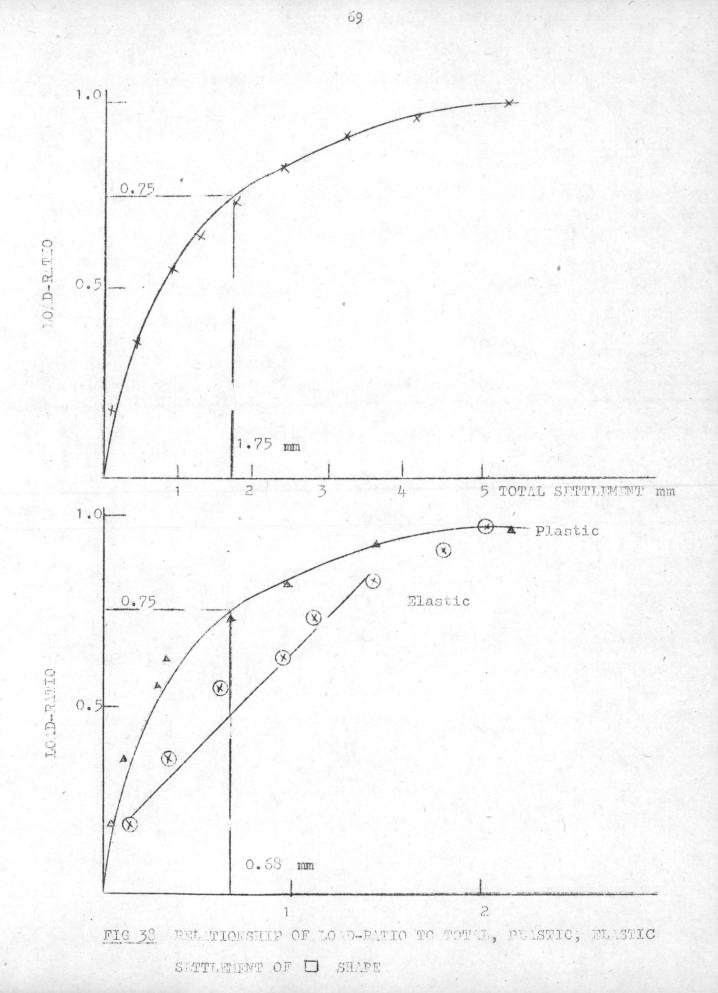


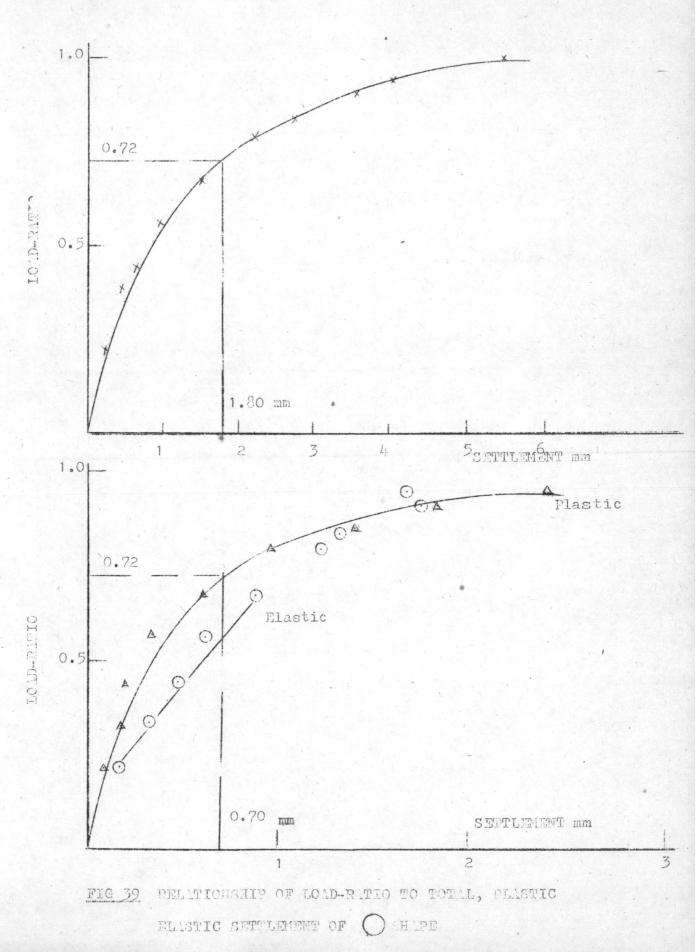


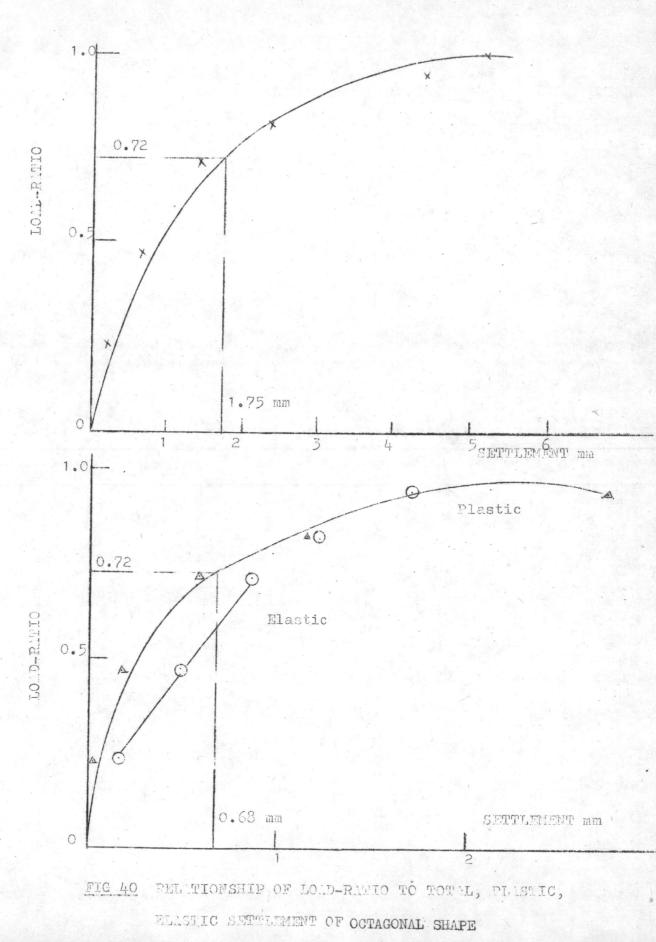


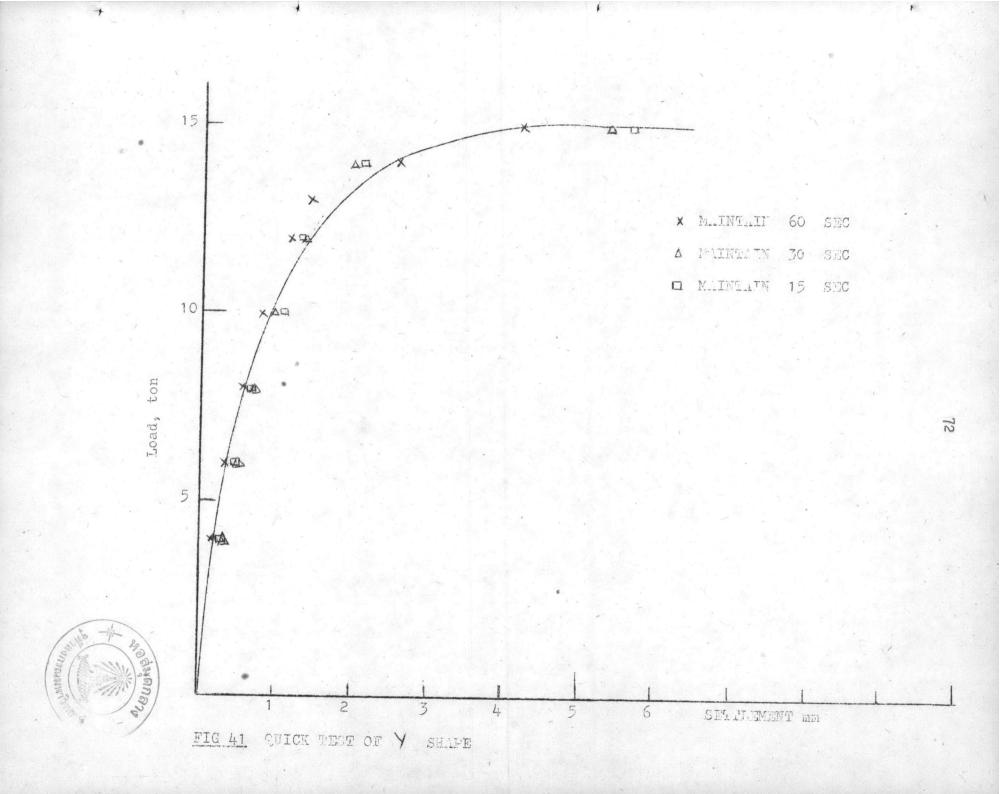
SETTLEMENT OF 23 SHAPE

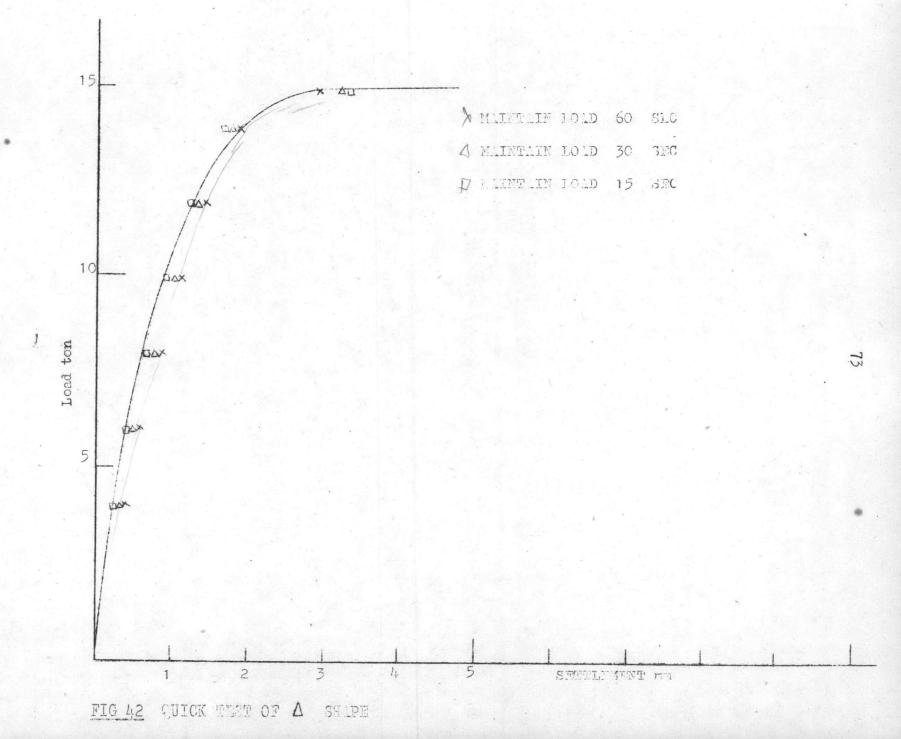
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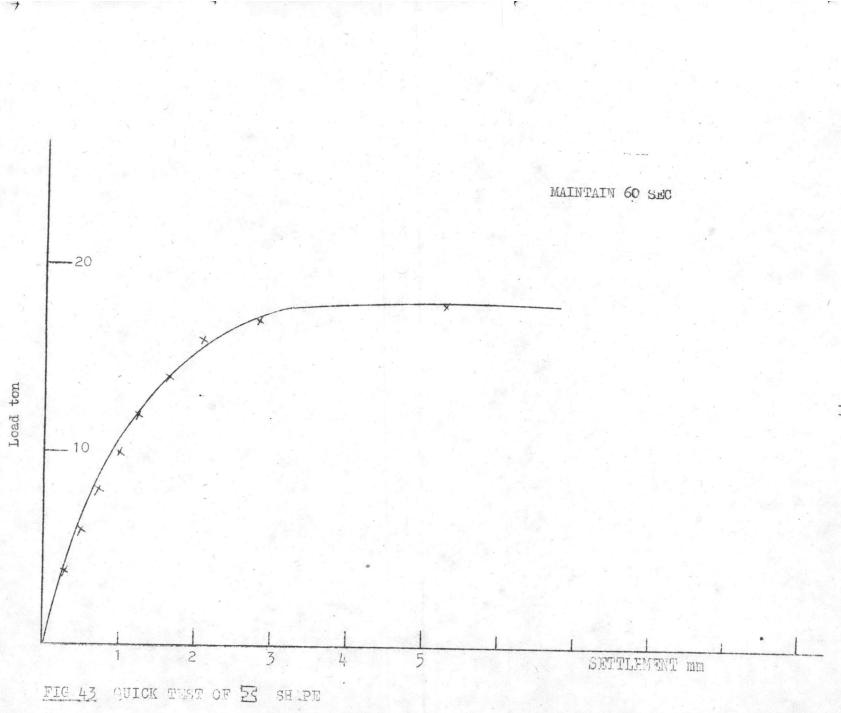


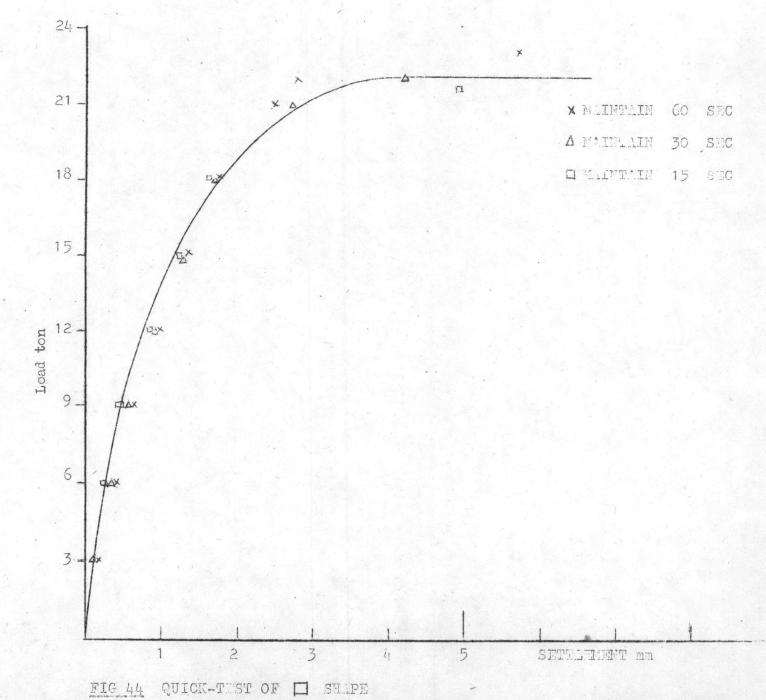








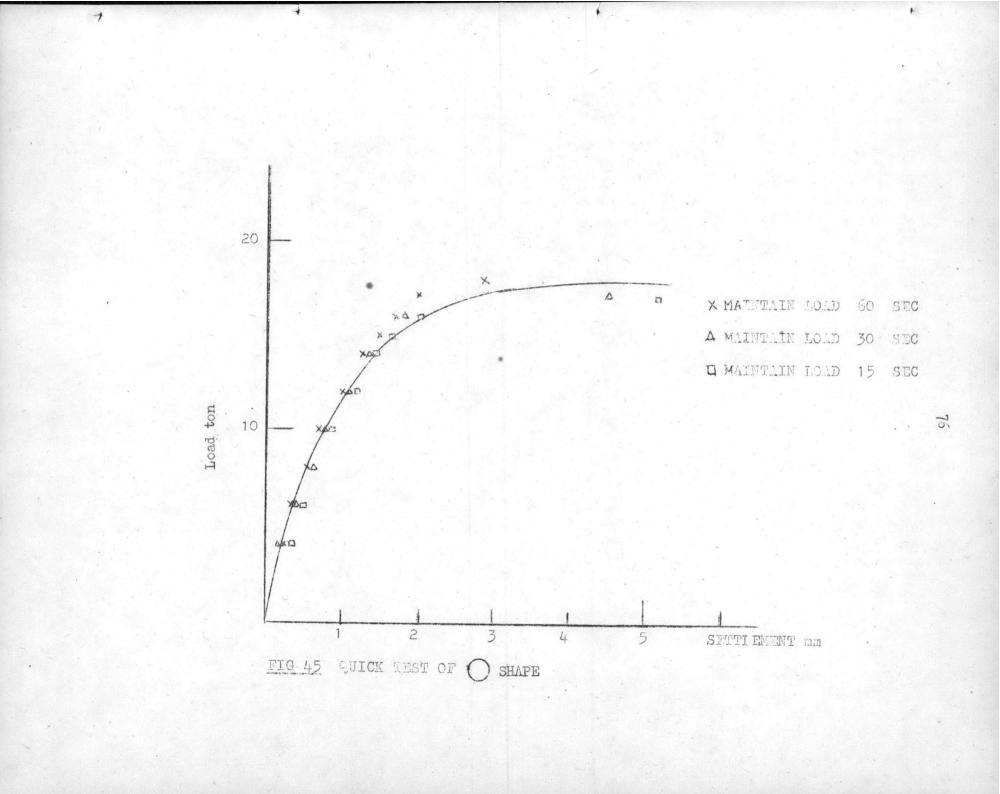


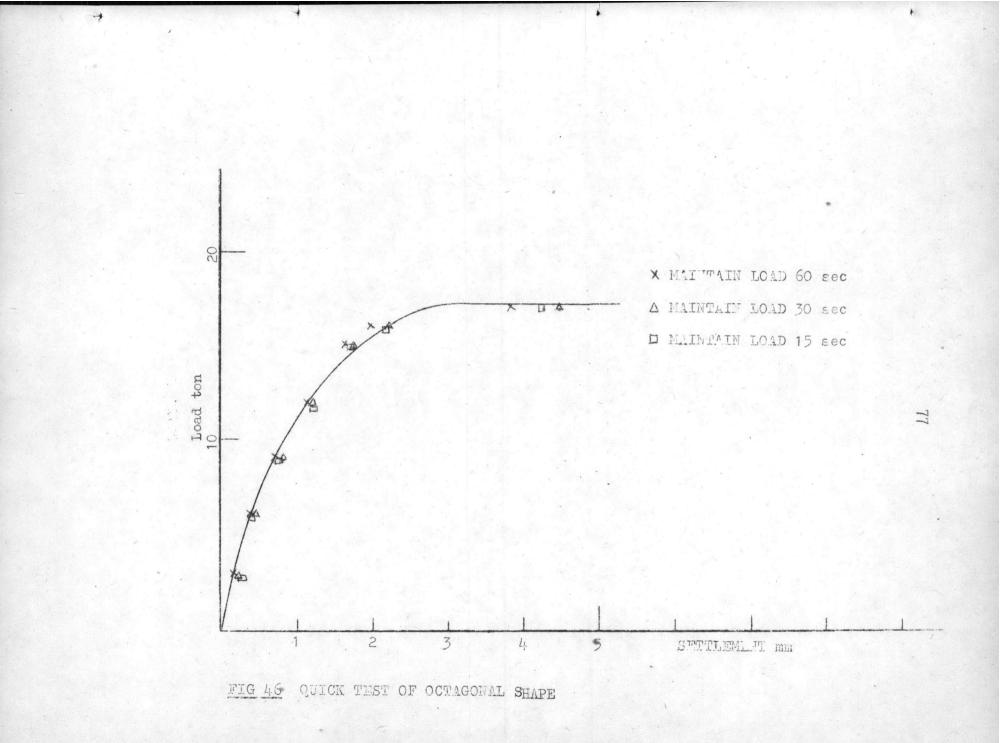


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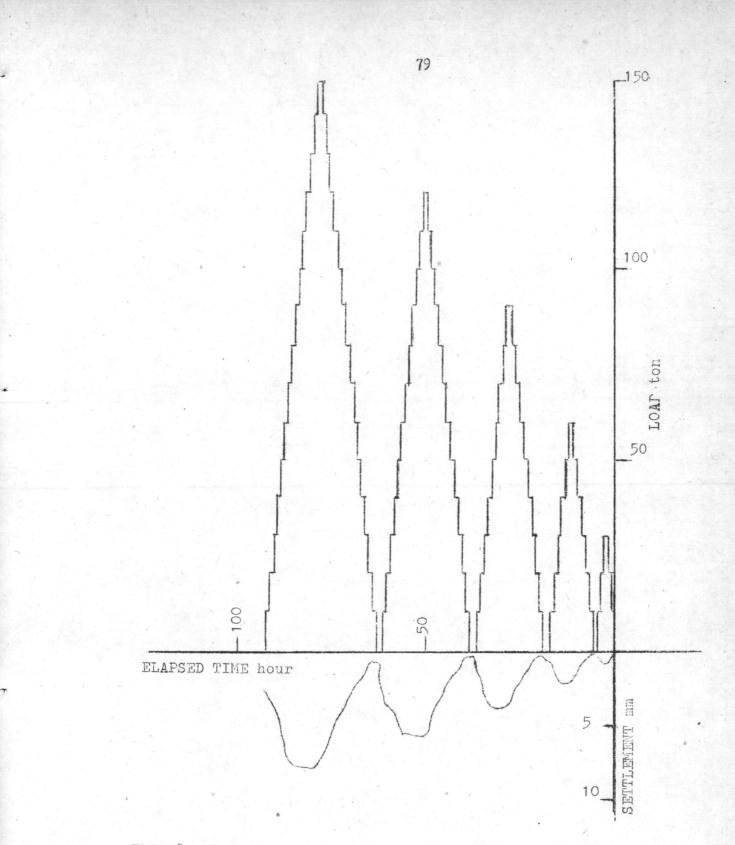


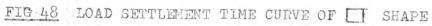


## SOIL AND MATERIAL LABORATORY BORING LOG

SCALE	LEGEND	LOCATION NONG KHEAM BORING No. BH-1 GROUND ELEVATION	FIELD MOISTURE CONTENT 20, 40, 60, 80	BLOWS PER FOOT 20 40 60 80	UNCONFINED COMPRESSIVE STRENGTH kg/cm <sup>2</sup> 1, 2, 3, 4	IN SITU-VANE SHEAR STRENGTH Kg/cm <sup>2</sup>
M FT	211	NOT A STATE OF A DESCRIPTION OF A DAMAGE AND THE OWNER				
-2		TOP SOIL 0.70 m.	100	•	<del>0.15 0.114</del>	0.345
_ 4			100		0.12 0.114	0.229
	-> 	Very soft dark grey clay with bit of shell.	94 \$		o.ii 0.ii4	0.229
		city with the of onesi.	86		0.18 0.114	0.229
-8_	· · ·		82 0		020 0.114	***0:229 *
- 10 -	*		61		029 0.114	0.229
- 12 -			58			0.458
<del>.</del> : -		SOft dark grey clay.	° 54			Undisturbed
- 14 			¢ 53	94	9 1.12	Remolder
- 16 -  - 18 _		Stiff to very stiff greyish brown clay and some peat.	• 30	20	1.26	
- 20 -		Dense to very dense	23	41	91.75	1 
- 22 -		brownish coarse sand	917	68/9"	··· · ·	
- 24`` -	A 6 0 4 6 10, 60	and gravel.	• 16 • 17	62/6" 0 70/7" 0		
	1	Bottom of hole 25.0 m.				
	1.			•	1. <b>1</b> .	

FIG 47 SOIL PROPERTIES AT NONG KHAEM





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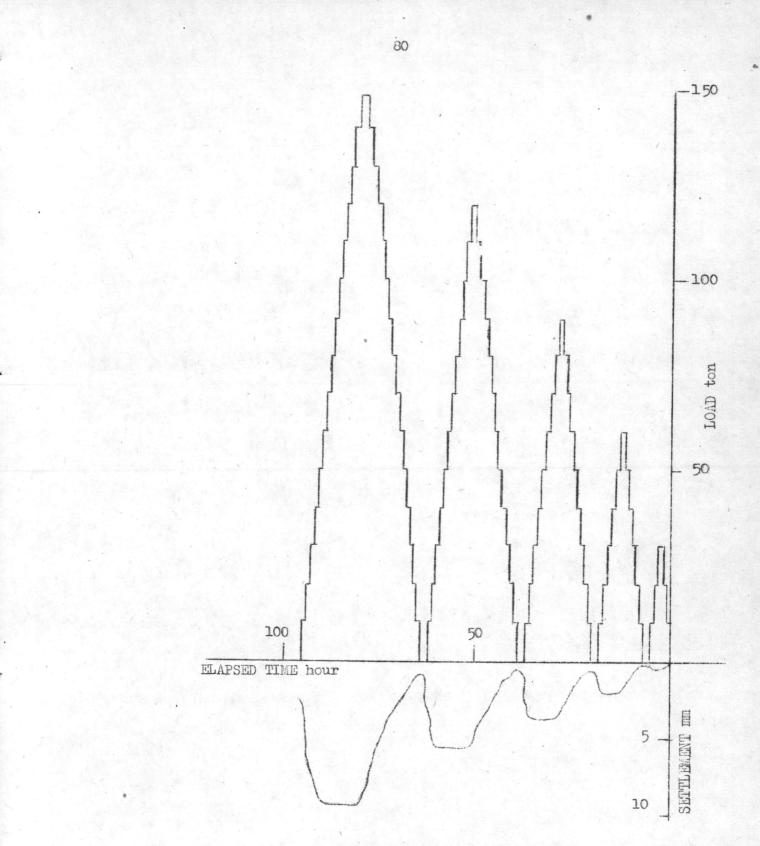
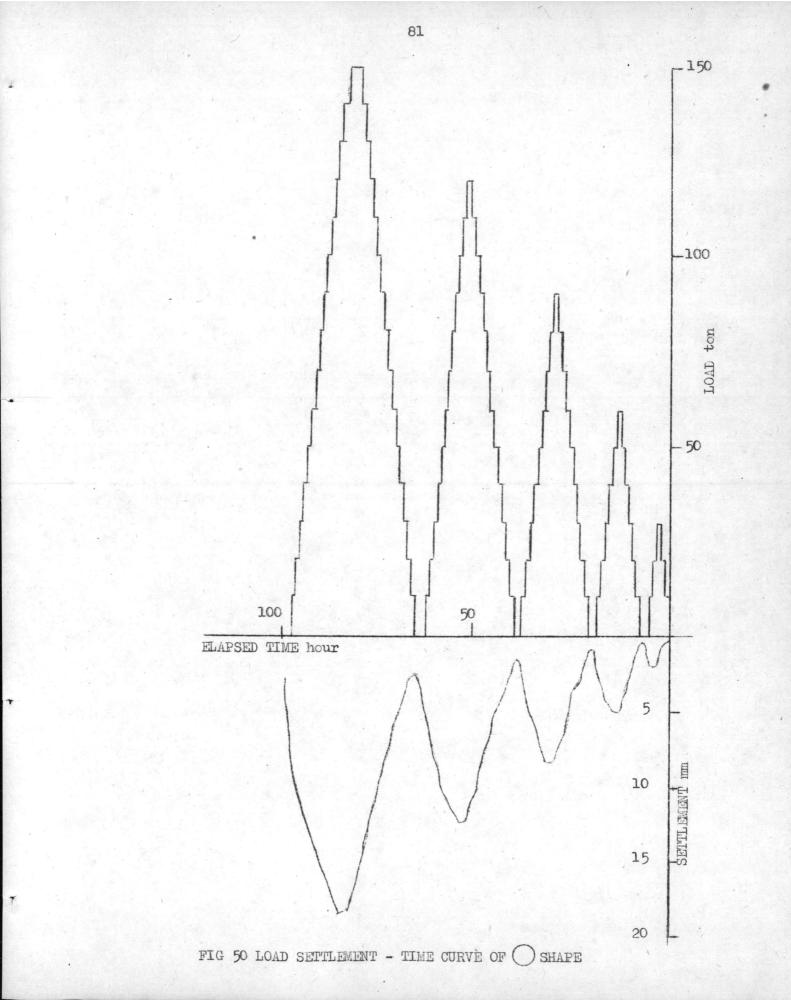
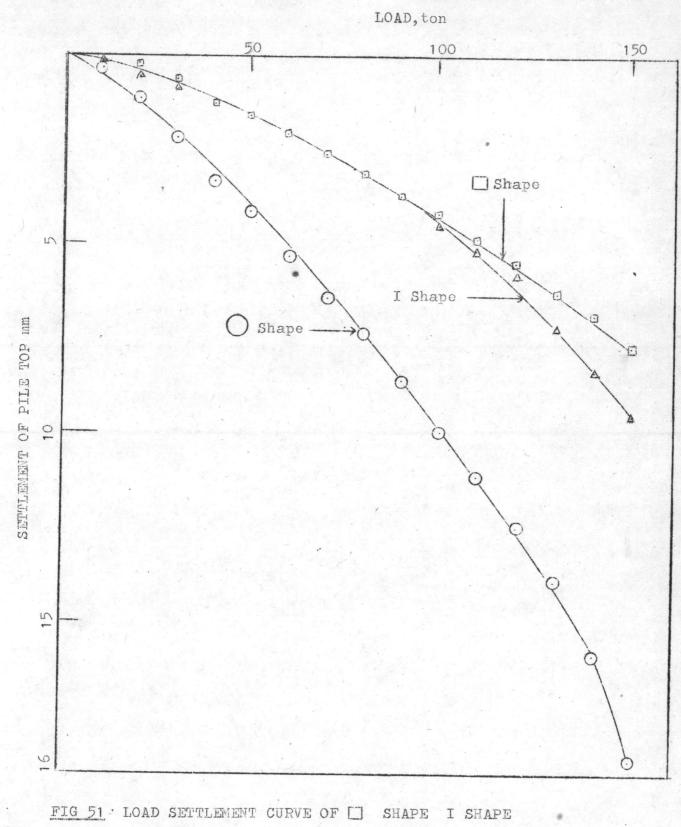


FIG 49 LOAD SETTLEMENT - TIME CURVE OF I SHAPE

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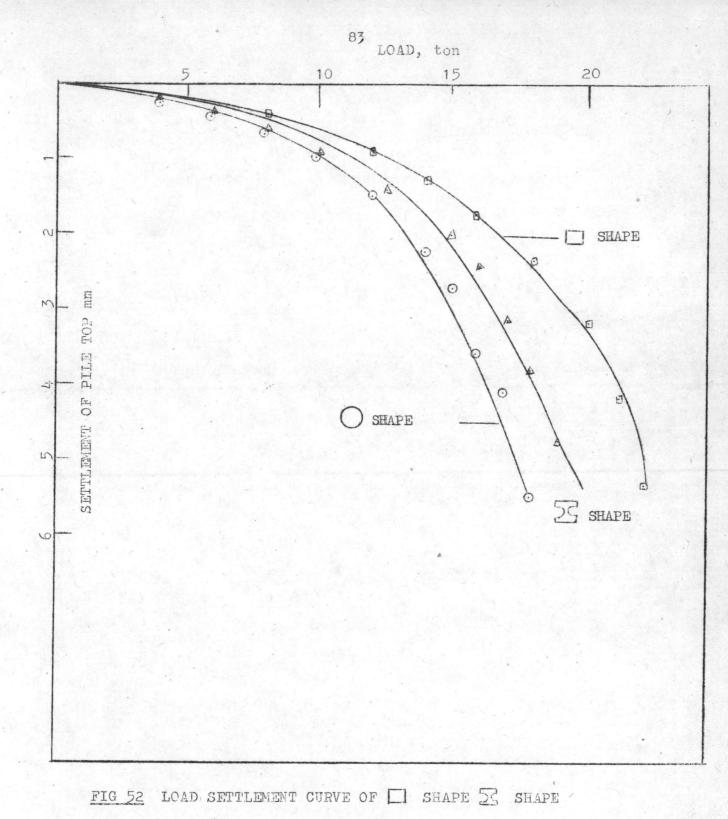




AND O SHAPE (LONG PILE)

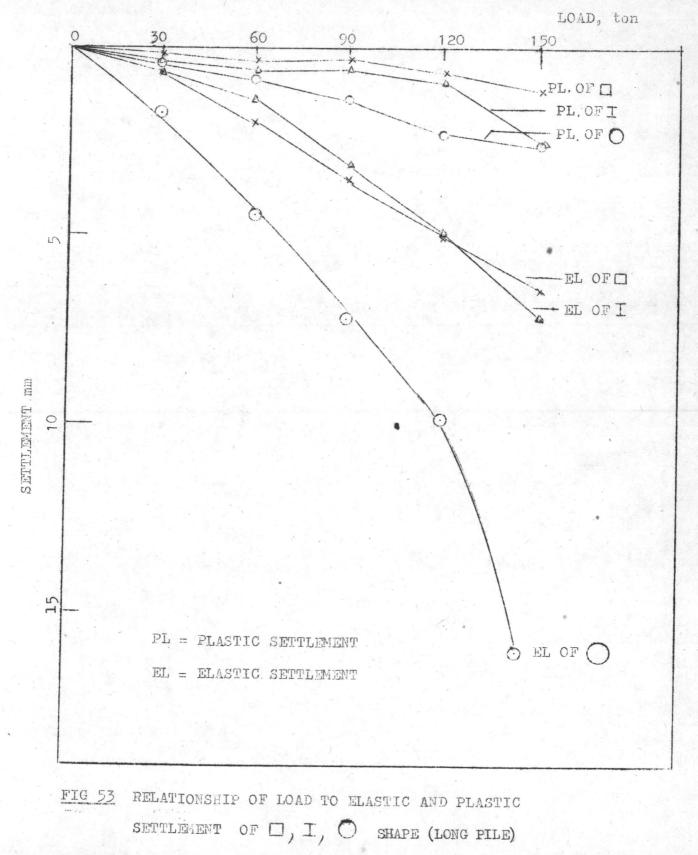
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AND O SHAPE (SHORT PILE)

X



Section		$ \Delta $	53		$\odot$	0
Test after driving (days)	10	13	12	18	12	19
Embeded length (m)	9.87	9.87	9.84	9.80	10.35	9.87
Perimeter around boundary (cm)	114	105	169	140	110	115
Minimum perimeter (cm)	107.5	105	144	140	110	116
Cross sectional area (cm <sup>2</sup> )	512	530	929	1225	785	1015
Skin area/end area	219	195	179	112	118	113
Perimeter/area ratio (cm)	0.22	0.198	0.182	0.114	0.140	0.114
Failure load (ton)	15	15	19	22	.18	17
Stress at failure load (kg/cm <sup>2</sup> )	29.3	28.3	20.4	17.9	22.9	16.7
Settlement at failure load (mm)	4.50	4.40	4.80	5.30	5.50	5.20
Failure load/volume ton/m <sup>3</sup>	29.3	28.3	20.5	18.0	21.8	16.7
Settlement at 0.5 failure load (mm)	0.75	0.75	0.80	0.80	0.85	0.75
Settlement at 0.75 failure load (mm)	1.5.	1.5	1.75	1.95	2.00	1.75

<u>Note</u> \* End area of circular pile =  $962 \text{ cm}^2$ 

Pile  $\oint 6" - 6 m$  skin area / end area = 160

TABLE 2 SUMMARY OF LOAD-SETTLEMENT OF ALL PILES

Section	57	$\triangle$	53		$\odot$	$\bigcirc$
Maximum plastic settlement (mm)	240	247	275	311	240	270
Maxium elastic settlement (mm)	193	197	205	225	169	171
At intercept between plastic and						
elastic curve settlement (mm)	1.8	1.92	2.02	2.00	1.82	1.46
load (ton)	14.7	15	18.6	21	16.2	15

TABLE 3 SUMMARY RELATIONSHIP BETWEEN LOAD AND PLASTIC, ELASTIC SETTLEMENT

Load ratio at the maximum curvature of the plastic-load ratio curve	0.8	0.75	0.73	0.75	0.72	0.72
Tatal settlement at the max curvature (mm)		1.000		1.1.1.	1.80	
Plastic settlement at the max curvature (mm)		1			0.70	1.1.1.1
Elastic settlement at the max curvature (mm)			1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	1.55	1.10	
Load at the max curvature (ton)	12	11.25	13.87	16.50	12.96	12.24

TABLE 4 SUMMARY RELATIONSHIP BETWEEN LOAD-RATIO AND TOTAL, PLASTIC ELASTIC SETTLEMENT.

Failure load from ML test	15	15	19	22	18	17
failure load from quick test .	15	15	19	22	17.5	17
Difference	0	0	0	0	2.8%	0

TABLE 5 COMPARISON ML TEST AND QUICK TEST OF ALL PILES

Section	$\bigtriangledown$	$\triangle$	53		$\bigcirc$	$\bigcirc$
End area cm <sup>2</sup>	512	530	929	1225	962	1015
Skin area						
(based on minimum perimeter) m <sup>2</sup>	10.61	10.36	14.16	13.72	11.38	11.44
Failure load ton	15	15		22	18	17
End load (N_=9, vane shear) ton	1.04	1,08		in the second second		
End load ( $N_c = 9$ , basd on $U/C$ ) ton	0.76			1.83	N	
Unit skin friction (Based on vane						
shear) ton/m <sup>2</sup>	1.32	1.34	1.21	1.42	1.41	1.30
Unit skin friction (Based on U/C)				- San San		
ton/m <sup>2</sup>	1.34	1.37	1.24	1.47	1.46	1.35
Adhesion factor (Based on vane						
shear)	0.53	0.54	0.48	0.57	0.56	0.52
Adhesion factor (Based on U/C)		0.83		A second second		
Adhesion factor (Tomlinson)						
Upper limit base on vane shear	0.92	0.92	0.92	0.92	0.92	0.92
base on U/C	1.05	N 1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.05	
Lower limit base on vane shear	0.50	0.50			0.50	
base on U/C	0.60	0.60	1.11	0.60	2 1 2 2 C 2 1	0.60
Adhesion factor (Holmberg)						
base on vane shear	0.82	0.82	0.82	0.82	0.82	0.82
base on U/C	1.00	1.00	1.00	1.00	1.00	1.00
Unconfined compression :- Average sh End shear <u>Vane shear test</u> :- Average sh End shear	lear st streng ear st	rength th rength	l. l.	66 to 66 to 50 to	n/m <sup>2</sup> n/m <sup>2</sup> n/m <sup>2</sup> n/m <sup>2</sup>	1.0

TABLE 6 SUMMARY THE RELATIONSHIP OF THE BEARING CAPACITY OF PILES TO THE PROPERTIES OF SOIL