การรับแรงในแนวราบของ เสา เข็มในคืนกรุง เทพฯ



นายศุภชัย สิทธิเลิศประสิทธิ์

005072

วิทยานิพนธ์นี้ เป็นส่วนหนึ่งของการศึกษาตามหลักสูตรปริญญาวิศวกรรมศาสตรมหาบัณฑิต แผนกวิชาวิศวกรรมโยธา บัณฑิตวิทยาลัย จุฬาลงกรณมหาวิทยาลัย

พ.ศ. 2519

"The Laterally Loaded Piles in Bangkok Clay"

Mr. Suphachai Sithilertprasit

A Thesis Submitted in Partial Fulfillment of the Requirements

for the Degree of Master of Engineering

Department of Civil Engineering

Graduate School

Chulalongkorn University

1976

Accepted by the Graduate School, Chulalongkorn University in partial fulfillments of the requirement for the Degree of Master of Engineering.

(Prof. Dr. Visid Prachuabmoh)

Dean

Thesis Committee

Niwat Daranande Chairman

(Prof. Dr. Niwat Daranandana)

(Prof. Dr. Chai Muktabhant)

V. Tengammuy Member

(Associate Prof. Vichien Tengamnuay)

S. Burnag Member

(Assist. Prof. Dr. Supradit Bunnag)

Thesis Advisor: Prof. Dr. Chai Muktabhant

Copyright 1976

by

The Graduate School Chulalongkorn University

Thesis Title "The Laterally Loaded Piles in Bangkok Clay"

By Mr. Suphachai Sithilertprasit

Department of Civil Engineering.

Thesis Title The Laterally Loaded Piles in Bangkok Clay.

Name Mr. Suphachai Sithilertprasit

Department of Civil Engineering

Academic Year 1976

ABSTRACT

A programme of pile test was carried out at Chulalongkorn University campus, where there is a deep deposit of soft clay, to examine the performance of laterally loaded unrestrained piles. Short rectangular piles of 18 x 35 cm.

3.50 m. long and long piles of 18 cm. square 7 m. long were employed. Maintained load (ML), constant rate of deflection (CRD), quick maintained load and repetitive loading tests were carried out on the piles.

The ultimate lateral resistance has been calculated by Brom's method assuming that failure takes place either when one plastic hinge form along each individual pile or when the lateral resistance of the supporting soil is exceeded along the total length of the laterally loaded pile. Lateral deflections at working loads have been calculated using the theory of subgrade reaction and the theory of elasticity.

The results from the proposed design methods have been compared with test data. Satisfactory agreement has been found between measured and calculated ultimate lateral resistance. The results confirm that the subgrade reaction theory greatly overestimates the lateral deflection of the pile foundation, however good agreement is found between

the measured lateral deflections and the predicted lateral deflections, using a value of Young's modulus of 400 $\rm c_u$ in the analysis.

หัวข้อวิทยานิพนธ์ การรับแรงในแนวราบของเสาเข็มในคินกรุงเทพฯ ชื่อ นายศุภซัย สิทธิเลิศประสิทธิ์ แผนกวิชาวิศวกรรมโยธา ปีการศึกษา 2519

บทคัดยอ

การทคสอบการรับแรงของเสาเข็มในแนวราบไค้กระทำในบริเวณ

จุฬาลงกรณมหาวิทยาลัย เสาเข็มที่ใช้ทคสอบเป็นเสาเข็มคอนกรีศชนิคที่หัวเข็มสามารถ
เคลื่อนที่โดยอิสระ เสาชนิคสั้นเป็นรูปสี่เหลี่ยมผืนผ้าขนาค 18 x 35 เซ็นติเมตร

ยาว 3.50 เมตร เสาชนิคยาวเป็นรูปสี่เหลี่ยมจัตุรัส ขนาค 18 x 18 เซ็นติเมตร

ยาว 7 เมตร วิธีที่ใช้ในการทคสอบมี maintained load test, constant rate of deflection test, quick maintained load and repetitive loading tests.

การคำนวณหาคาแรงตานทานสูงสุดตามแนวราบ (ultimate lateral resistance) ใช้วิธีของบรอมส์ (BROMS) โดยถือว่าเสาเซ็มถึงชั้นพิบัติ เมื่อเกิดจุดหมุนพลาสติก (plastic hinge) ขึ้น 1 จุด ในเสาเซ็ม หรือเมื่อดิน ตลอดความยาวของเสาเซ็มไม่สามารถรับแรงได้อีก เมื่อแรงยังอยู่ในพิกัดใช้งานระยะ โกงตัวในแนวราบของเสาเซ็มคำนวณได้จาก Theory of subgrade reaction และทฤษฎีการยืดหยุ่น (Theory of elasticity)

จากการเปรียบเทียบผลการทคสอบกับทฤษฎี ปรากฏว่าค่าแรงต้านทาน สูงสุคตามแนวราบจากการทคลองมีค่าใกล้เคียงกับการคำนวณ

จากการทคลองนี้ยังชี้ให้เห็นอีกว่า Subgrade reaction theory
จะให้คาระยะโก่งตัวของเสาเข็มเกินกว่าความเป็นจริง แต่อย่างไรก็ดีเมื่อใช้คา
โมคูลัสย็คหยุ่น (Modulus of elasticity) ของคินเท่ากับ 400 c
นในการคำนวณจะให้คาระยะโก่งตัวของเสาเข็มใกล้เคียงกับความจริง

ACKNOWLEDGEMENTS

The research was carried on at the Department of
Civil Engineering, Chulalongkorn University and was financed
by a University research grant.

The author would like to express his gratitude to his advisor, Prof. Dr. Chai Muktabhant for his suggestion and encouragement throughout the course of the research. He also wishes to thank Prof. Dr. Niwat Daranandana, Associate Prof. Vichien Tengamnuay and Assist. Prof. Dr. Supradit Bunnag for serving as the thesis committee.

This research project was made possible under the sponsor of the Concrete Products & Aggregate Co., LTD. The author also wishes to thank Dr. Panitan Lukkunaprasit and Mr. Suebsak Promboon for their suggestions and Mr. Pipat Pithyachariyakul for his help in preparing the equipments for test in this research.

TABLE OF CONTENTS

CHAPTER		TITLE	PAGE
	Titl	e Page	. i
	Thes	is Approval	. iii
	Abst	ract	. iv
	Ackn	owledgements	. vii
	Tabl	e of Contents	. viii
	List	of Tables	. xi
	List	of Figures	. xii
	List	of Symbols	. xiv
· I	INTR	ODUCT ION	. 1
II	LITE	RATURE REVIEW	. 3
	2.1	Behavior at Working Loads	. 3
		2.1.1 Theory of Beam on Elastic	
		Foundation	4
		2.1.2 Theory of Subgrade Reaction	5
	2.2	Ultimate Lateral Resistance	23
		2.2:1 General	23
		2.2.2 Unrestrained Piles	24
	2.3	Lateral Load Tests	31
		2.3.1 Types of Load Test	31
		2.3.2 Criteria of Failure	34
111	EXPE	RIMENTAL INVESTIGATION	37
	3.1	Soil Sampling and Field Vane Testing	37
	3.2	Laboratory Testing	37



HAPTER			TITLE	PAGE
HAPIER .				
	3.3	Descri	ption of the Piles	39
		3.3.1	Details of Piles ····································	39
		3.3.2	Pile Location and Layout Plan	39
		3.3.3	Procedure of Testings	40
		3.3.4	Pile Material Properties ······	41
		3.3.5	Programme of Pile Tests	43
IV	ANAL	YSIS OF	RESULTS	48
	4.1	Young'	s Modulus of Soil	48
	4.2	Pile C	lassifications	49
	4.3	Coeffi	cient of Subgrade Reaction	51
		4.3.1	Short Piles	51
		4.3.2	Long Piles	52
	4.4	Load-D	eflection Characteristics	52
		4.4.1	Prediction of Load-Deflection	
			Relationship at Working Load by	
			Subgrade Reaction Theory	52
		4.4.2	Prediction of Load-Deflection	
			Relationship at Working Load by	
			Theory of Elasticity	51
		4.4.3	Prediction of Deflections and Be	nding
			Moments along the Long Pile belo	w the
			Ground Surface	50
		4.4.4	Load-Deflection at Ground Surfac	e 5'
		4.4.5	Effect of Repeated Loading	5'

CHAPTER	TITLE	PAGE
	4.5 Ultimate Lateral Resistance	57
	4.5.1 Prediction of Ultimate Lateral	
	Resistance of Files by Brom's	
	Method	57
	4.5.2 Ultimate Lateral Resistance	79
V	DISCUSSION	84
	5.1 Modulus of Elasticity of Soil and	
	Coefficient of Subgrade Reaction	84
	5.2 Load-Deflection Curves	84
	5.3 Effect of Repeated Loading	87
	5.4 Ultimate Lateral Resistance	88
VI	CONCLUSION AND RECOMMENDATION FOR FUTURE	
	STUDY	90
	REFERENCES	94
	APPENDIX A	104
	APPENDIX B	120
	APPENDIX C	122
	APPENDIX D	125
	VITA	127

LIST OF TABLES

TABLE	TITLE	PAGE
1	Evaluation of the Coefficient n ₁	12
2	Evaluation of the Coefficient n2	13
3	Numerical Value of Coefficient m	13
4	Programme of Pile Tests	47
5	Coefficient of Subgrade Reaction of Short Pile.	51
6	Coefficient of Subgrade Reaction of Long Pile	52
7	Lateral Deflection (E _s = 18.43 ksc.)	74
8	Lateral Deflection (E _s = 104 ksc.) · · · · · · · · · · · · · · · · · · ·	76
9	Ultimate Load (Pile No. 1)	80
10	Ultimate Load (Pile No. 2)	81
11	Ultimate Load (Pile No. 3)	82
12	Ultimate Load (Pile No. 4)	82
13	Ultimate Load (Pile Nos. 5 and 6)	83

A STANDARD OF THE STANDARD OF

LIST OF FIGURES

FIGURE	TITLE	AGE
1	Distribution of Soil Reactions	11
2	Calculation of Lateral Deflections for a Short	
	Pile	14
3	Failure Modes for Free-Headed Piles	24
4	Distribution of Lateral Earth Pressures	25
5	Deflection, Soil Reaction and Bending Moment	
	Distribution (a Short Free-Headed Pile)	27
6	Deflection, Soil Reaction and Bending Moment	
	Distribution (a Long Free-Headed File)	29
7	Soil Properties at the Test Site	38
8	The Details of a Short Pile	1,4
9	The Details of a Long Pile	45
10	Layout of Test Piles	46
11	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 1, ML Test)	58
12	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 1, CRD Test)	59
13	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 1, Quick Test)	60
14	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 2, ML Test)	61
15	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 2. CRD Test)	62

FIGURE	TITLE	PAGE
16	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 2, Quick Test)	63
17	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 3, ML Test)	64
18	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 3, CRD Test)	65
19	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 3, Quick Test).	66
20	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 4, ML Test)	67
21	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 4, CRD Test)	68
22	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 4, Quick Test)	69
23	Comparisons Between Observed and Computed	
	Load-Deflection Curves (Pile No. 5, ML Test)	70
24	Comparisons Between Observed and Computed	
	Load-Deflection Curves (File No. 6, ML Test)	71
25	Load-Deflection Relationship from Repeated	
	Loading Test (Pile No. 2)	72
26	Load-Deflection Relationship from Repeated	
	Loading Test (Pile No. 3)	73

LIST OF SYMBOLS

The following symbols have been adopted for use in this paper:

 $A(x) = e^{-\beta x} (\cos \beta x + \sin \beta x)$

B = diameter or width of load plate, in cm.

 $B(x) = e^{-\beta x} \sin \beta x$

 $C(x) = e^{-\beta x}(\cos \beta x - \sin \beta x)$

cu = cohesion determined from undrained triaxial, direct shear or vane tests, in ksc.

D = diameter or width of test pile, in cm.

 $D(x) = e^{-\beta x} (\cos \beta x - \sin \beta x)$

d = deflection of load plate, in cm.

E_p = modulus of elasticity of pile material, in ksc.

E_s = modulus of elasticity of soil, in ksc.

E₅₀ = secant modulus corresponding to nalf the ultimate unconfined compressive strength of the soil, in ksc.

f = distance from 1.5 pile diameters below ground surface to location of maximum bending moment in cm.

g = distance from location of maximum bending moment to bottom of pile in cm.

Ip = moment of inertia of pile section, in cm4.

K = kD, in ksc.

 $K_0 = k_0 B$, in ksc.

 $K_{\infty} = k_{\infty} D$, in ksc.

k = coefficient of subgrade reaction, in kg/cm³

k_m = coefficient of lateral subgrade reaction with respect
to moment acting at mid-height of short laterally
loaded piles, in kg/cm³

k_o = coefficient of subgrade reaction of square or circular plates, in kg/cm³

k
p = coefficient of lateral subgrade reaction with respect
to load acting at mid height of short laterally
loaded piles, in kg/cm³

k_∞ = coefficient of lateral subgrade reaction for a long laterally loaded pile, in kg/cm³

L = length of embedment, in cm.

L' = equivalent length of embedment, in cm.

n₁ = coefficient

n₂ = coefficient

M = moment, in kg-cm

pos = maximum positive bending moment, in kg-cm

Mmax = maximum negative bending moment, in kg-cm

 $M_{\mbox{yield}}$ = yield or ultimate moment resistance of pile section, in kg-cm

P = Lateral load, in kg.

P.1+ = ultimate lateral resistance, in kg.

Q = soil reaction per unit length of pile, in kg/cm

q = unit soil reaction, in ksc.

q, = unconfined compressive strength, in ksc.

qult = ultimate lateral resistance, in ksc.

U.C. = unconfined compression test

W₁ = liquid limit

W_n = natural water content

W_n = plastic limit

y = lateral deflection, in cm.

y = lateral deflection at ground surface, in cm.

ym = lateral deflection at ground surface caused by moment acting at mid-height of short piles, in cm.

y_p = deflection at ground surface caused by load acting at mid-height of short piles, in cm.

y_{calc} = calculated lateral deflection at ground surface, in cm.

ytest = measured lateral deflection at ground surface, in cm.

of = coefficient equal to n₁ n₂

BL = dimensionless length

Ms = poisson's ratio